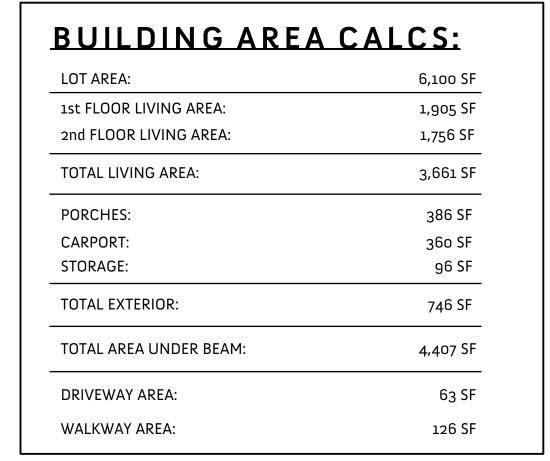
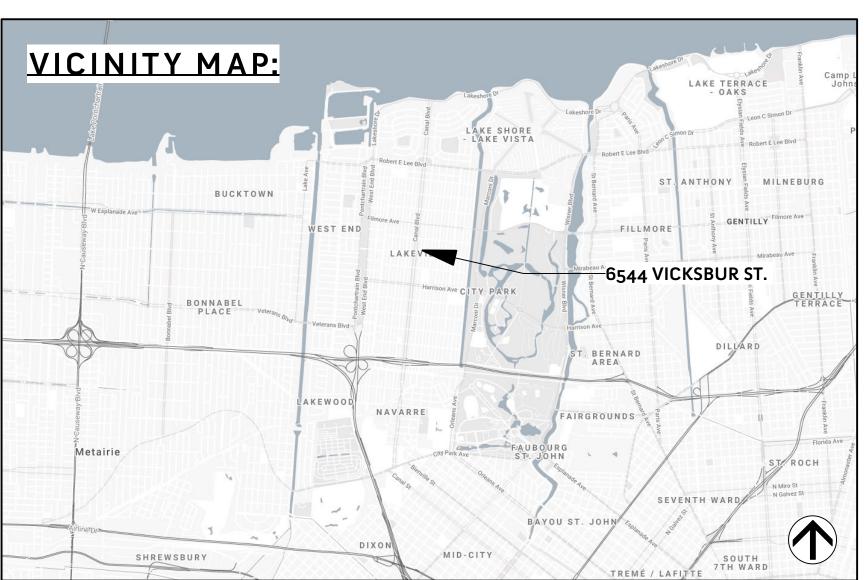
## SINGLE FAMILY RESIDENCE:

6544 VICKSBURG ST.
LAKEVIEW SUBDIVISION
SQUARE 178 LOT 37 & 38

# NEW ORLEANS, LA ZONING DIST: S-LRS1



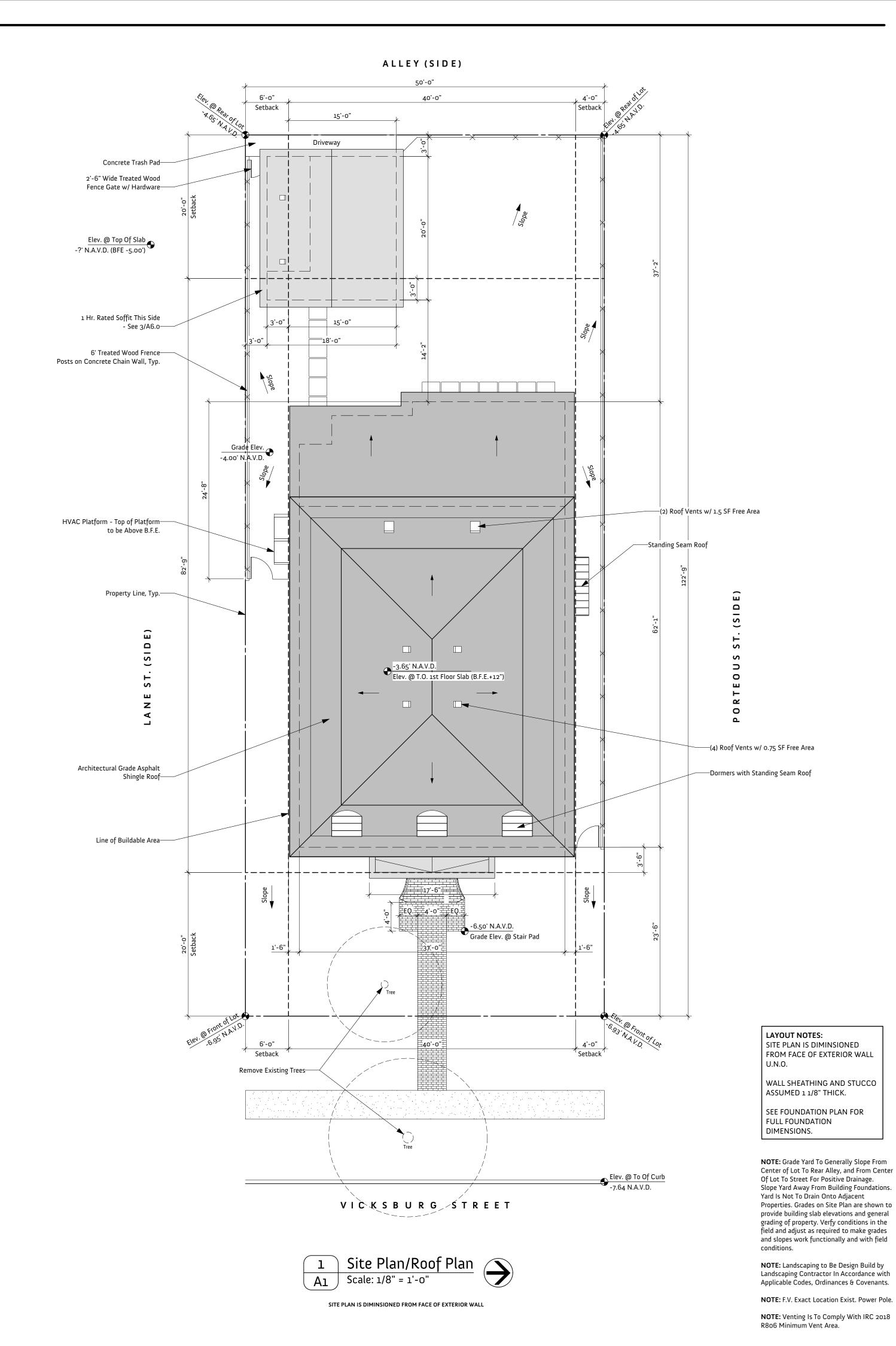


#### **DRAWING INDEX:** Cover Sheet & Site Plan Floor Plans Schedules Framing Plans Roof Framing Plan RCP/Elec. Plans **Exterior Elevations Exterior Elevations** Wall Sections Window Flashing Details S1.oB General Notes S1.1 Pile Plan Post Tension Plan S1.2 S1.3 Foundation Plan Foundation Sections S1.4 Foundation Sections Framing Details Carport Foundation Plans S2.0 Carport Foundation Sections and Details

## **GENERAL NOTES:**

- 1. Dimensions shown on the Floor Plan are face of stud to face of face of stud unless noted otherwise.
- 2. Contractor shall verify and coordinate clear dimensions required for equipment furnished by others and casework which is located in new work.
- 3. Contractor shall not hang any equipment and/or materials from the HVAC ducts. All suspended equipment shall be hung from the structure above. Contractor shall not puncture HVAC ducts with sprinkler pipes or any other building materials.
- 4. Plumbing fixture locations are shown on the floor plans. Plumbing system is to be Design/Build by the Plumbing contractor in accordance with the 2015 International Plumbing Code.
- 5. HVAC system to be Design Build by the Mechanical Contractor in accordance with the 2015 International Mechanical Code. Additionally, outside air (at the rate of .05 CFM per Sq.Ft. of conditioned space) shall be introduced at the return plenum with sufficient means to balance to the required air flow.
- 6. Electrical panel location, outlet & lighting locations shown on floor plan. Electrical system is to be Design/Build by the electrical contractor in accordance with the 2014 National Electrical Code.
- 7. Coordinate with Owner regarding selection of specific finishes.
- 8. General Contractor shall coordinate, apply & pay for all permits, inspections and/or certifications from the appropriate agencies.
- 9. General Contractor shall be responsible for all means, methods, techniques, sequences, procedures and required safety precautions to accomplish the work.
- 10. All roof & exterior wall penetrations shall be flashed and waterproofed per applicable codes and manufacturer's standards and recommendations.
- 11. All materials, systems and building components shall be installed in accordance with applicable codes and manufacturer's standards and recommendations.

  12. Window protection is to be provided per IRC Section R301.2.1.2 with wood (1/2" plywood) protective panels w/attachment hardware to be attached per Table IRC Table R301.2.1.2.
- 13. Residence is to be constructed to withstand 130 MPH wind thrust in accordance with Section R301.2.1.4, IRC 2018. See Wall Straping Details & Load Tables For Continuous Load Path Construction on attached structural drawings.
- 14. Vapor barrier on top of soil under slab to be Stego Industries ASTM E 1745 Class A Vapor Barrier or approved equal. Install per manufacutrer's requirements.
- 15. All material shall be new and UL listed.
- 16. The contractor shall inform the Architect in writing, during the bidding period, of any discrepancies or omissions noted on the drawings or specifications or of any variations needed in order to conform to codes, rules and regulations. Upon receipt of such information, the Architect will send written instructions to all concerned. Any such discrepancy, omission, or variation not reported shall be the responsibility of the contractor, and work shall be performed in a manner as directed by the Architect.
- 17. Firestopping & draftstopping shall be per local building codes.
- 18. Termite protection shall be provided in compliance with Sec. R318 IRC 2018 ed., by EPA registered & labeled Chemical Termiticide treatment or Termite Baiting System, installed and maintained per manufacturer's requirements.
- 19. Provide & Install Cement Tile Backer Board At All Tub & Shower Surrounds.
- 20. All existing trees to remain shall be protected during construction to avoid damage from adjacent work, equipment and soil or fill being piled on or against root system & base.
- 21. No drywall products used for this project shall be manufactured in China or be from manufacturers known to be involved in ongoing litigation involving high sulphur levels. Contractor shall provide material data sheets to Owner for drywall products provided.





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> Single Family Resident 6544 Vicksburg St.

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06.17.22	Owner	DD Progress		
07.07.22	Owner	Design Development		
07.22.22	Owner	Construction Docs.		
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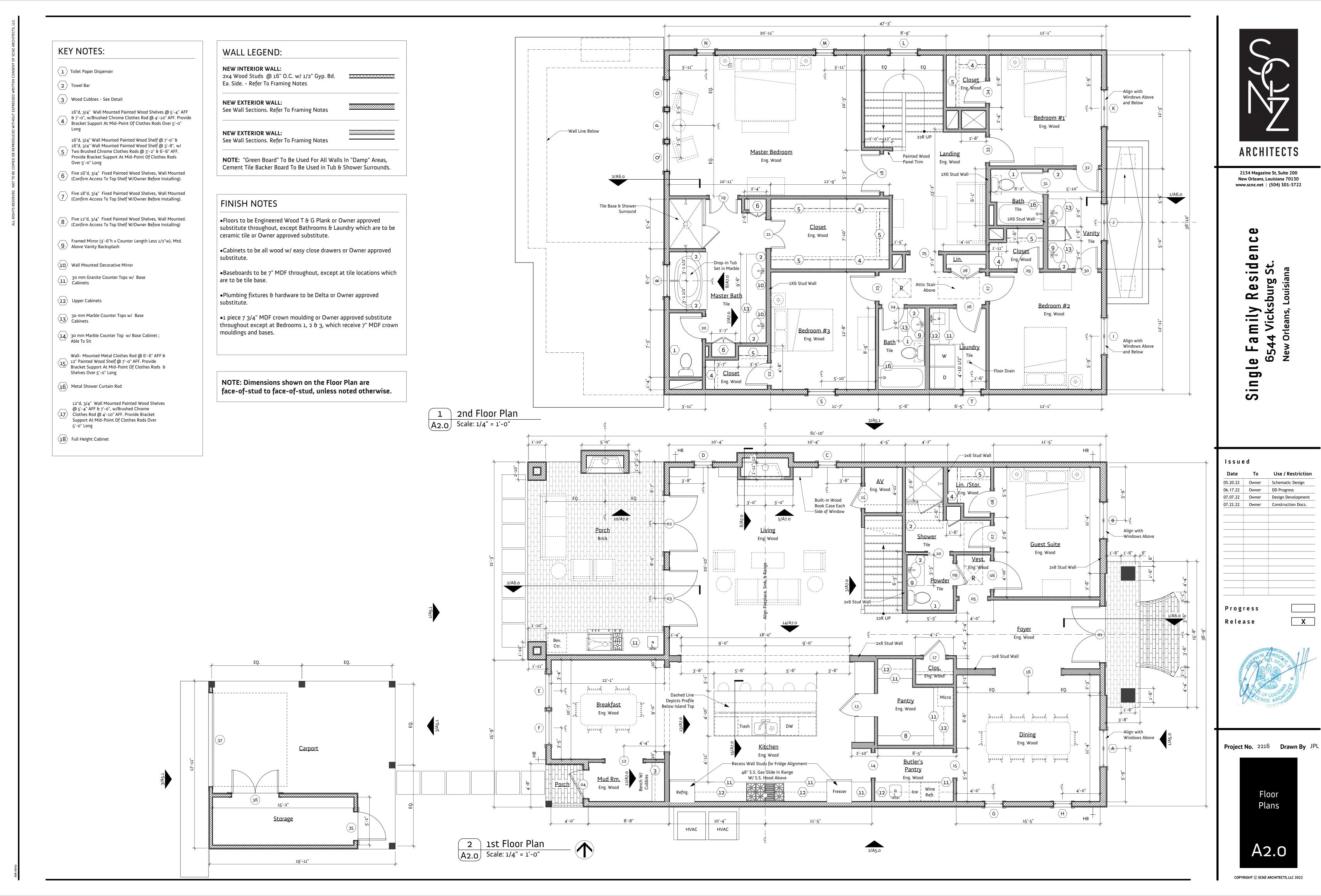
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Release



Project No. 2216 Drawn By JPL





ırk	Width	Height	Thickness	Operation	Lites		Remarks
L	6'o"	10'0"	1 3/4"	Swing Bi-part	See Elev.	Solid core wood panel door - exterior, with lites, radius top	
2	6'o"	8'o"	1 3/4"	Swing Bi-part	See Elev.	Solid core wood panel door - exterior, with lites, 1'-6" transom	
3	6'o"	8'o"	1 3/4"	Swing Bi-part	See Elev.	Solid core wood panel door - exterior, with lites, 1'-6" transom	
4	3'0"	8'0"	1 3/4"	Swing Simple	See Elev.	Solid core wood panel door - exterior, with lites	
)5	3'0"	8'0"	1 3/4"	Cased Opening			
06	3'0"	8'o"	1 3/4"	Swing Simple		2-panel, solid core, raised panel door	
07	2'4"	8'o"	1 3/4"	Swing Simple		2-panel, solid core, raised panel door	
08	2'4"	8'o"	1 3/4"	Swing Simple		2-panel, solid core, raised panel door	
9	2'4"	8'0"	1 3/4"	Swing Simple		2-panel, solid core, raised panel door	
10	2'4"	8'o"	1 3/4"	Pocket Simple		2-panel, solid core, raised panel door	
L1	2'4"	8'0"	1 3/4"	Swing Simple		2-panel, solid core, raised panel door	
12	4'0"	8'0"	1 3/4"	Cased Opening			
13	2'4"	8'o"	1 3/4"	Swing Simple		Custom wood door to match cabinets	
14	2'8"	8'o"	1 3/4"	Cased Opening			
15	2'8"	8'o"	1 3/4"	Cased Opening			
.6	7'0"	8'o"	1 3/4"	Cased Opening			
17	2'4"	8'o"	1 3/4"	Swing Simple		2-panel, solid core, raised panel door	
.8	4'0"	8'o"	1 3/4"	Swing Bi-part		2-panel, solid core, molded hard board raised panel door	
L <b>9</b>	3'0"	8'o"	1 3/4"	Swing Bi-part		Solid core, raised panel door	
20	2'4"	8'o"	1 3/4"	Swing Simple		Solid core, raised panel door	
21	3'0"	8'o"	1 3/4"	Swing Bi-part		Solid core, raised panel door	
22	2'4"	8'o"	1 3/4"	Swing Simple		2-panel, solid core, molded hard board raised panel door	
23	2'8"	8'o"	1 3/4"	Swing Simple		2-panel, solid core, molded hard board raised panel door	
24	2'4"	8'o"	1 3/4"	Swing Simple		2-panel, solid core, molded hard board raised panel door	
25	3'0"	8'o"	1 3/4"	Cased Opening		2-panel, solid core, molded hard board raised panel door	
26	2'8"	8'o"	1 3/4"	Swing Simple		2-panel, solid core, molded hard board raised panel door	
27	2'8"	8'o"	1 3/4"	Swing Simple		2-panel, solid core, molded hard board raised panel door	
28	3'0"	8'o"	1 3/4"	Swing Bi-part		2-panel, solid core, molded hard board raised panel door	
29	2'4"	8'o"	1 3/4"	Swing Simple		2-panel, solid core, molded hard board raised panel door	
30	2'4"	8'o"	1 3/4"	Swing Simple		2-panel, solid core, molded hard board raised panel door	
31	2'4"	8'o"	1 3/4"	Swing Simple		2-panel, solid core, molded hard board raised panel door	
32	2'4"	8'o"	1 3/4"	Swing Simple		2-panel, solid core, molded hard board raised panel door	
33	2'8"	8'o"	1 3/4"	Swing Simple		2-panel, solid core, molded hard board raised panel door	
34	2'4"	8'o"	1 3/4"	Swing Simple		2-panel, solid core, molded hard board raised panel door	
35	3'0"	8'o"	1 3/4"	Swing Simple	See Elev.	Solid core wood panel door - exterior	
36	5'0"	8'o"	1 3/4"	Swing Bi-part	See Elev.	Solid core wood panel door - exterior	
37	10'0"	8'o"	1 3/4"	Overhead	See Elev.	Steel Garage Door W/ Remote Automatic Operation	

WI	WINDOW SCHEDULE							
Mark	No. of Units	Width	Height	Head Height	Sash Operation	Description	Glazing	Remarks
Α	1	4'0"	6'3"	10'0"	Bi-parting Casement	Aluminum	dbl. pane, insul.	W/ 2'-0" Transom, See Elev.
В	1	4'0"	6'3"	10'0"	Bi-parting Casement	Aluminum	dbl. pane, insul.	W/ 2'-0" Transom, See Elev.
С	1	2'0"	6'o"	8'o"	Single Hung	Aluminum	dbl. pane, insul.	
D	1	2'0"	6'o"	8'o"	Single Hung	Aluminum	dbl. pane, insul.	
Е	1	3,8	6'o"	9'0"	Single Hung	Aluminum	dbl. pane, insul.	
F	1	3,8	6'o"	9'0"	Single Hung	Aluminum	dbl. pane, insul.	
G	1	2'0"	6'o"	8'o"	Single Hung	Aluminum	dbl. pane, insul.	
Н	1	2'0"	6'o"	8'o"	Single Hung	Aluminum	dbl. pane, insul.	
1	1	4'0"	5'6"	8'o"	Bi-parting Casement	Aluminum	dbl. pane, insul.	
J	1	4'0"	5'6"	8'o"	Bi-parting Casement	Aluminum	dbl. pane, insul.	
K	1	4'0"	5'6"	8'o"	Bi-parting Casement	Aluminum	dbl. pane, insul.	
L	1	3,0,	6'o"	8'o"	Single Hung	Aluminum	dbl. pane, insul.	
М	1	2'0"	6'o"	8'o"	Single Hung	Aluminum	dbl. pane, insul.	
N	1	2'0"	6'o"	8'o"	Single Hung	Aluminum	dbl. pane, insul.	
0	1	3,0,	5'0"	8'o"	Single Hung	Aluminum	dbl. pane, insul.	
Р	1	3,0,	5'0"	8'o"	Single Hung	Aluminum	dbl. pane, insul.	
Q	1	3,0,	5'0"	8'o"	Single Hung	Aluminum	dbl. pane, insul.	
R	1	3'6"	5'0"	8'o"	Single Hung	Aluminum	dbl. pane, insul.	
S	1	3,0,	6'o"	8'o"	Single Hung	Aluminum	dbl. pane, insul.	
Т	1	2'0"	6'0"	8'o"	Single Hung	Aluminum	dbl. pane, insul.	
J	1	2'9"	3'3"	5'0"	Fixed Glass	Aluminum	dbl. pane, insul.	Attic Dormer with Arched Top, See Elevations
V	1	2'9"	3'3"	5'0"	Fixed Glass	Aluminum	dbl. pane, insul.	Attic Dormer with Arched Top, See Elevations
W	1	2'9"	3'3"	5'0"	Fixed Glass	Aluminum	dbl. pane, insul.	Attic Dormer with Arched Top, See Elevations
Dom	arkei							

#### Remarks:

Refer to Building Elevations for all door light muntin patterns and panel configurations.

- 2. Contractor is to field verify all measurements for doors to be inserted into existing openings prior to ordering.
- 3. Window R to Receive Safety Glazing in Compliance w/ IRC.R 308.3



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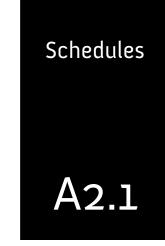
Date	To	Use / Restriction
05.20.22	Owner	Schematic Design
06.17.22	Owner	DD Progress
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07.22.22	Owner	Construction Docs.

Progress





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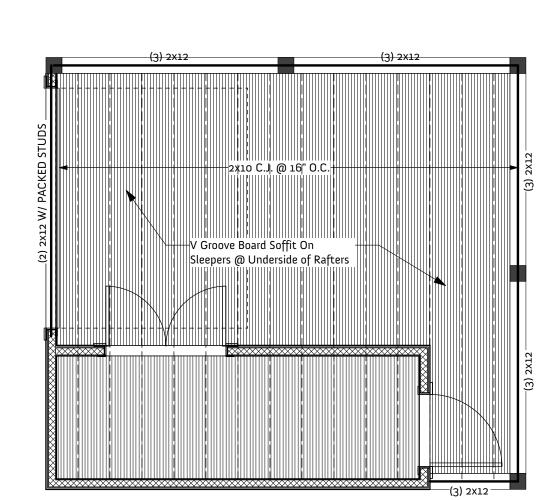


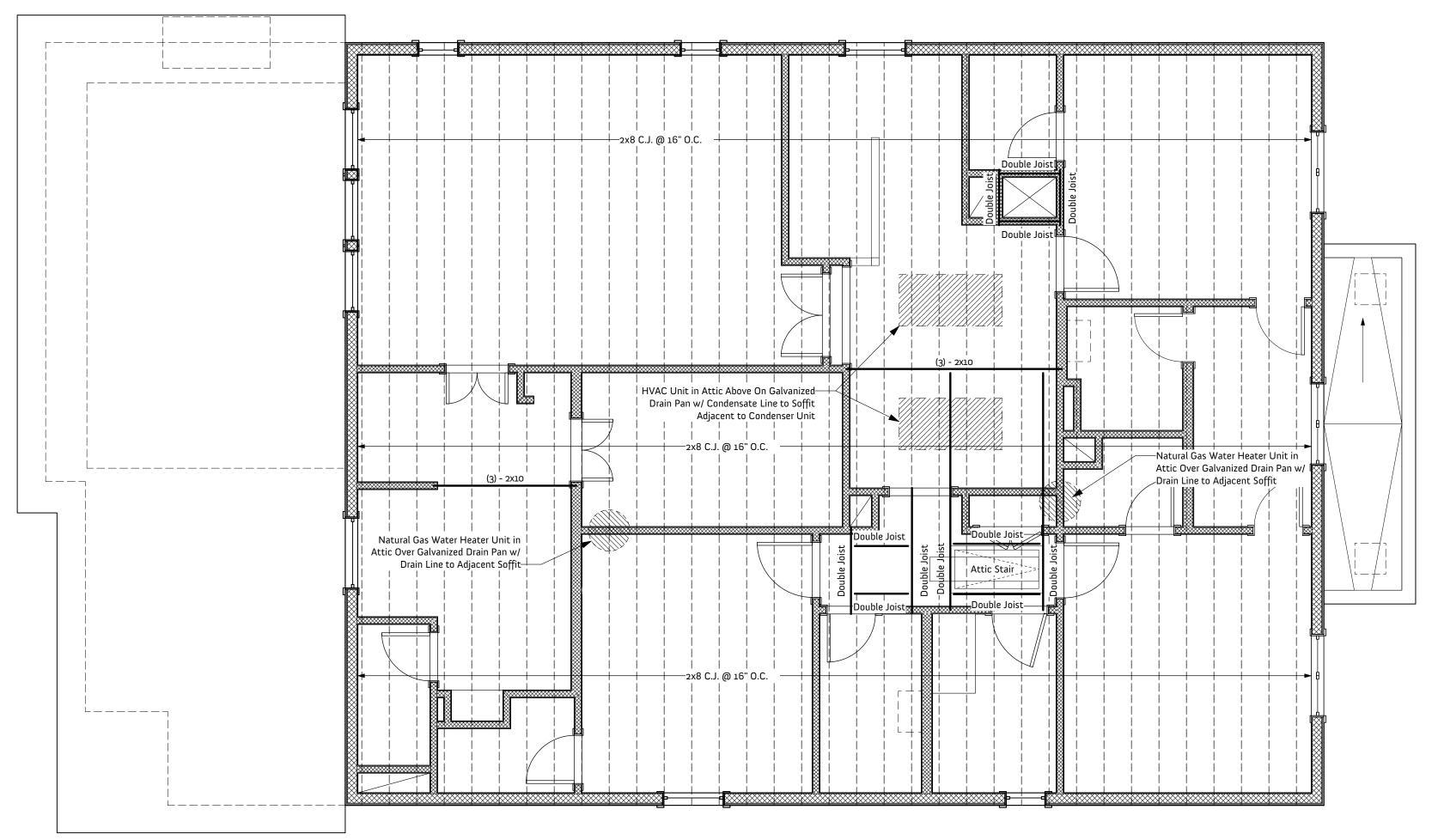
## FRAMING NOTES:

- ALL WOOD FRAMING FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE 2015 INTERNATIONAL RESIDENTIAL CODE (IRC) AND SHALL CONFORM TO THE WOOD FRAME CONSTRUCTION MANUAL (WFCM) FOR ONE- AND TWO-FAMILY DWELLINGS, 2001 EDITION AND THE PLYWOOD DESIGN SPECIFICATION BY THE APA.
- REFER TO STRUCTURAL DRAWINGS FOR WOOD FRAMING NOTES.
- UNLESS NOTED OTHERWISE MULTIPLE PIECES OF LUMBER OR MANUFACTURED WOOD PRODUCTS USED TO FORM BEAM OR HEADER MEMBERS SHALL BE ATTACHED TOGETHER WITH 2 ROWS OF 12d NAILS SPACED AT 12" FOR PIECES UP TO 12" DEEP. ALL OTHER PIECES SHALL HAVE 3 ROWS OF 12d NAILS AT 12".
- UNLESS SHOWN OTHERWISE ALL OPENINGS IN WALLS SHALL HAVE HEADERS CONSISTING OF A MINIMUM OF TWO 2X12'S OR THREE 2X10'S.
- PROVIDE DOUBLE FLOOR JOISTS UNDER ALL WALLS.
- PROVIDE BRIDGING FOR ALL FLOOR JOISTS @ 8' O.C. MAX.
- ALL LUMBER, PLYWOOD, LVL'S, OR OTHER STRUCTURAL WOOD ELEMENTS SHALL BE PRESSURE TREATED WITH ACQ TO A MINIMUM RETENTION OF 0.40 LBS/CU. FT. IN ACCORDANCE WITH AWPA. ALL LUMBER IN CONTACT WITH CONCRETE OR MASONRY SHALL BE TREATED.
- ALL WOOD MEMBERS (INCLUDING PLYWOOD SHEATHING OR BRACING) SHALL BE CONNECTED OR FASTENED WITH STEEL NAILS, SCREWS, OR BOLTS. NO STAPLES WILL BE PERMITTED. ALL WOOD CONNECTIONS SHALL BE IN ACCORDANCE WITH THE FASTENING SCHEDULE OF THE 2015 IRC, UNLESS NOTED OTHERWISE.
- UNLESS SHOWN OTHERWISE ALL PLYWOOD FLOOR SHEATHING SHALL BE APA RATED 48/24, 3/4" THICK AND FASTENED WITH GLUE AND 10d COMMON NAILS (.148" MIN. DIA. X 3" LG) OR SIMPSON WSNTL2 SCREWS (OR APPROVED EQUAL) SPACED AT 6" O.C. MAX ALONG SUPPORTING MEMBERS AT THE EDGES OF EACH SHEET AND 12" O.C. MAX ALONG SUPPORTING MEMBERS ON THE INTERIOR OF EACH SHEET.
- ALL MEMBERS DESIGNATED AS "LVL" SHALL BE LAMINATED VENEER LUMBER HAVING PROPERTIES AND STRENGTHS EQUAL TO THE i-LEVELTRUS JOIST COMPANY'S "MICROLLAM".
- ALL MEMBERS DESIGNATED AS "PSL" SHALL BE PARALLEL STRAND LUMBER HAVING PROPERTIES AND STRENGTHS EQUAL TO THE i-LEVEL TRUS JOIST COMPANY'S "PARALLAM".
- ALL MEMBERS LABELLED AS "TJI" ARE "I" SHAPED WOOD JOIST ELEMENTS HAVING THE STANDARD DESIGNATION, PROPERTIES, AND STRENGTHS EQUAL TO THOSE PRODUCED BY THE i-LEVEL TRUS JOIST COMPANY.
- ALL STRUCTURAL MEMBERS FABRICATED BY THE i-LEVEL TRUS JOIST COMPANY SHALL BE ERECTED AND BRACED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS.
- FRAMER SHALL BOX OUT AS NECESSARY AND PROVIDE CLEAR OPENINGS WHERE RETURN AIR CHASES PASS THROUGH FLOORING/CEILINGS. OPENINGS SHALL BE FRAMED PER IRC 2015 SECTION R502.10. NOTIFY ARCHITECT IF THERE ARE ANY DISCREPANCIES OR VARIATINS NEEDED TO CONFORM TO CODES.
- FLOOR FRAMING FOR ALL EXTERIOR SPACES IS TO BE TREATED WOOD.

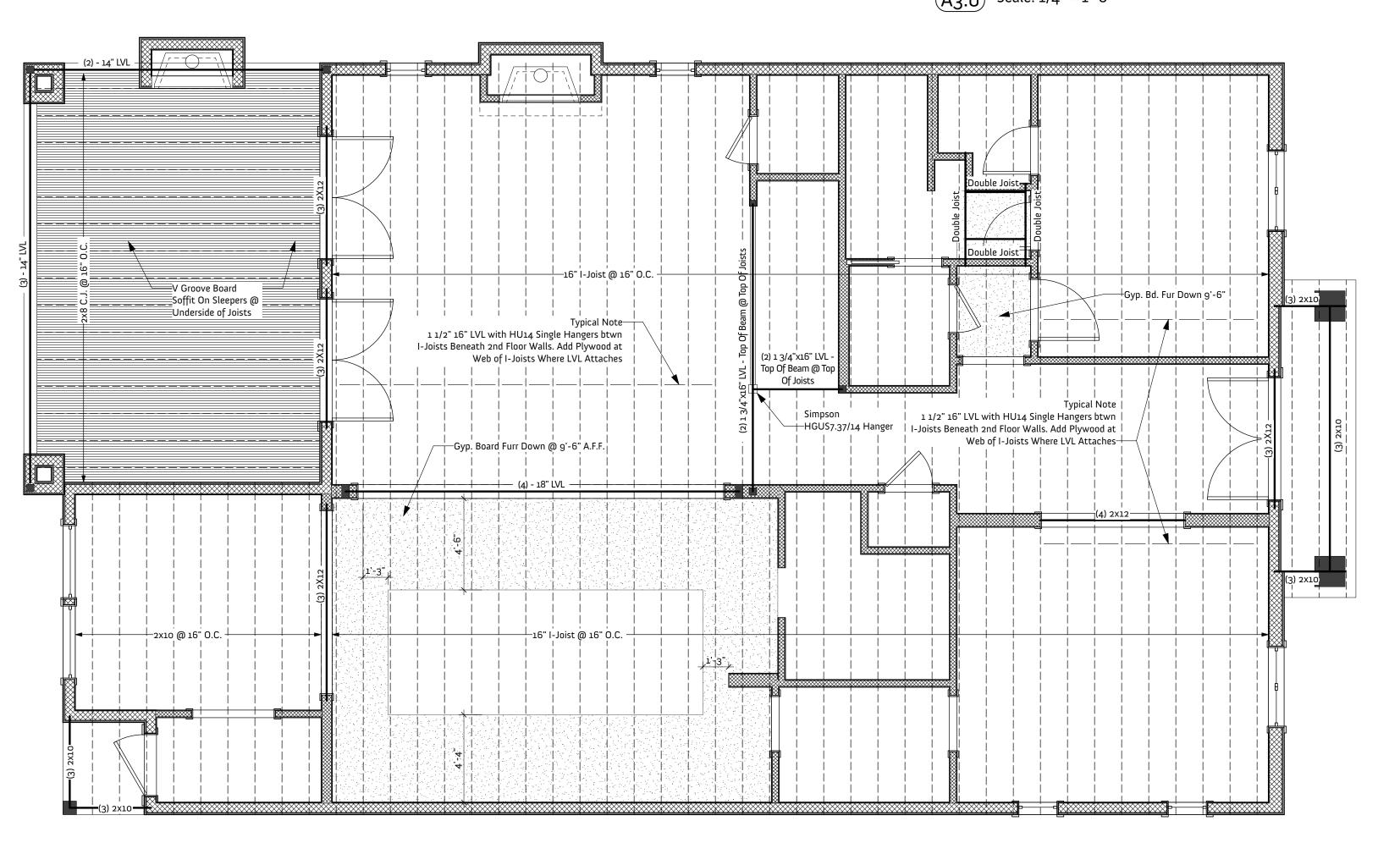
### **CEILING NOTES:**

- 1. CEILINGS TO BE 1/2" PAINTED GYP. BD., EXCEPT FOR BATHROOMS, WHICH ARE TO BE 1/2"
- 2. CEILINGS ARE TO BE MOUNTED TO UNDERSIDE OF JOISTS, U.N.O.
- SEE WALL SECTIONS FOR SOFFIT & EAVE DETAILS.
- NO FURRED CEILINGS UNLESS OTHERWISE NOTED OR SHOWN ON CEILINGS PLAN.





2nd Floor Ceiling Framing Plan
A3.0 Scale: 1/4" = 1'-0"



2 1st Floor Ceiling Framing Plan A3.0 Scale: 1/4" = 1'-0"



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06.17.22	Owner	DD Progress		
07.07.22	Owner	Design Developmer		
07.22.22	Owner	Construction Docs.		
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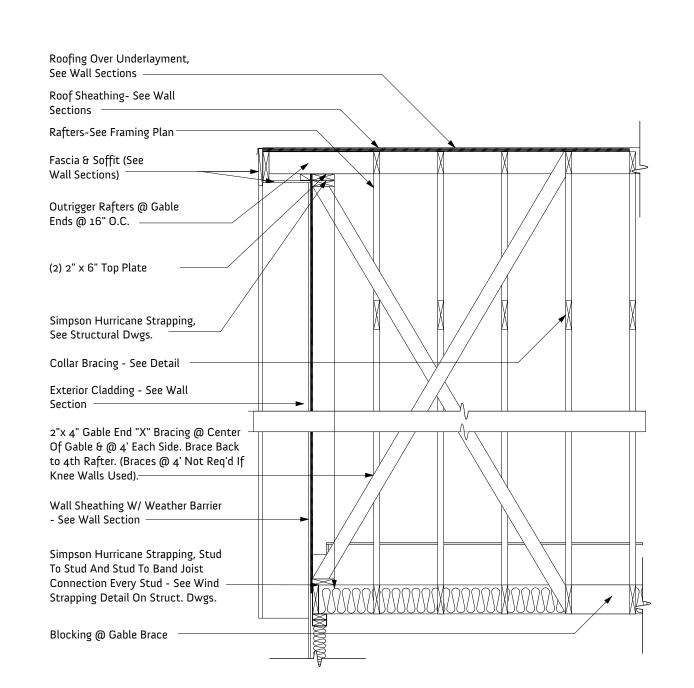


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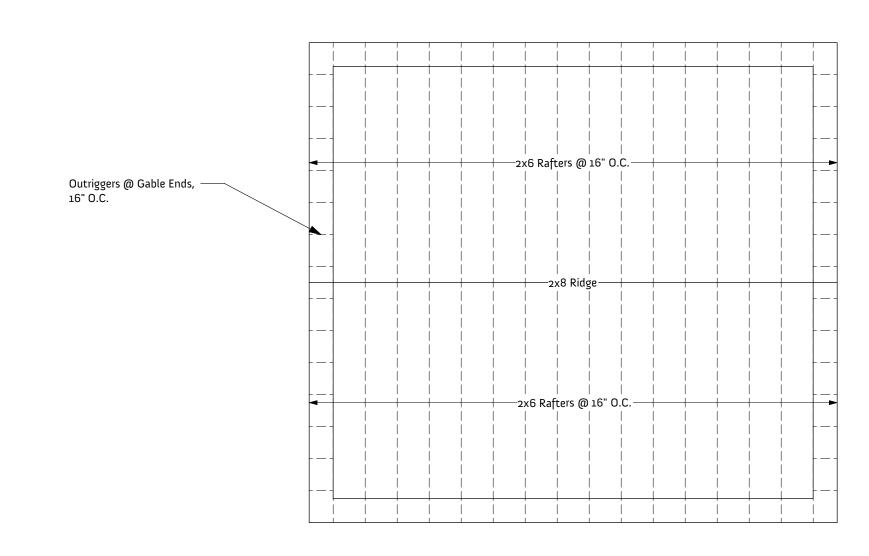


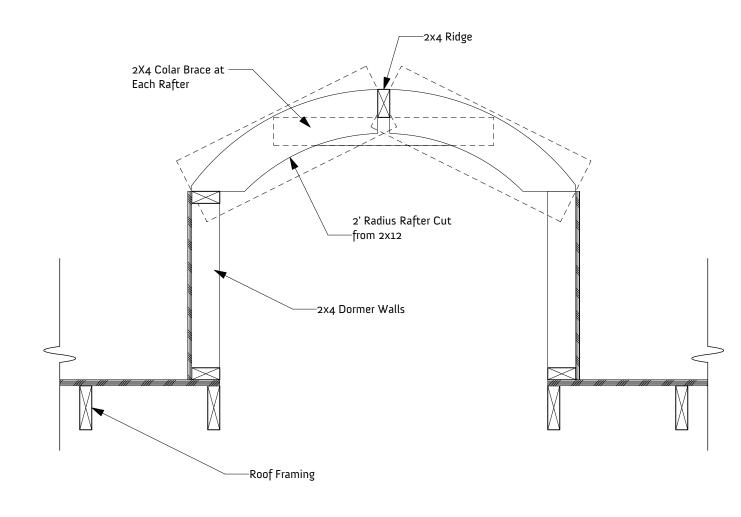
#### **FRAMING NOTES:**

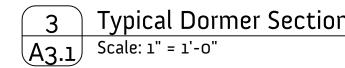
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- 9. UNLESS SHOWN OTHERWISE ALL PLYWOOD FLOOR SHEATHING SHALL BE APA RATED 48/24, 3/4" THICK AND FASTENED WITH GLUE AND 10d COMMON NAILS (.148" MIN. DIA. X 3" LG) OR SIMPSON WSNTL2 SCREWS (OR APPROVED EQUAL) SPACED AT 6" O.C. MAX ALONG SUPPORTING MEMBERS AT THE EDGES OF EACH SHEET AND 12" O.C. MAX ALONG SUPPORTING MEMBERS ON THE INTERIOR OF EACH SHEET.
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- 15. FLOOR FRAMING FOR ALL EXTERIOR SPACES IS TO BE TREATED WOOD.

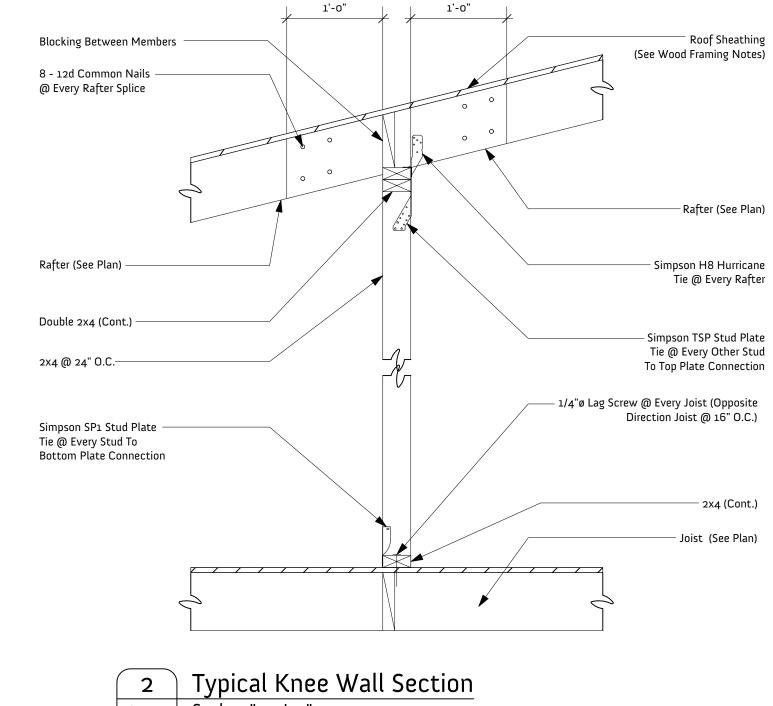


4 Gable Framing / Bracing Detail A3.1 Scale: 1/2" = 1'-0"

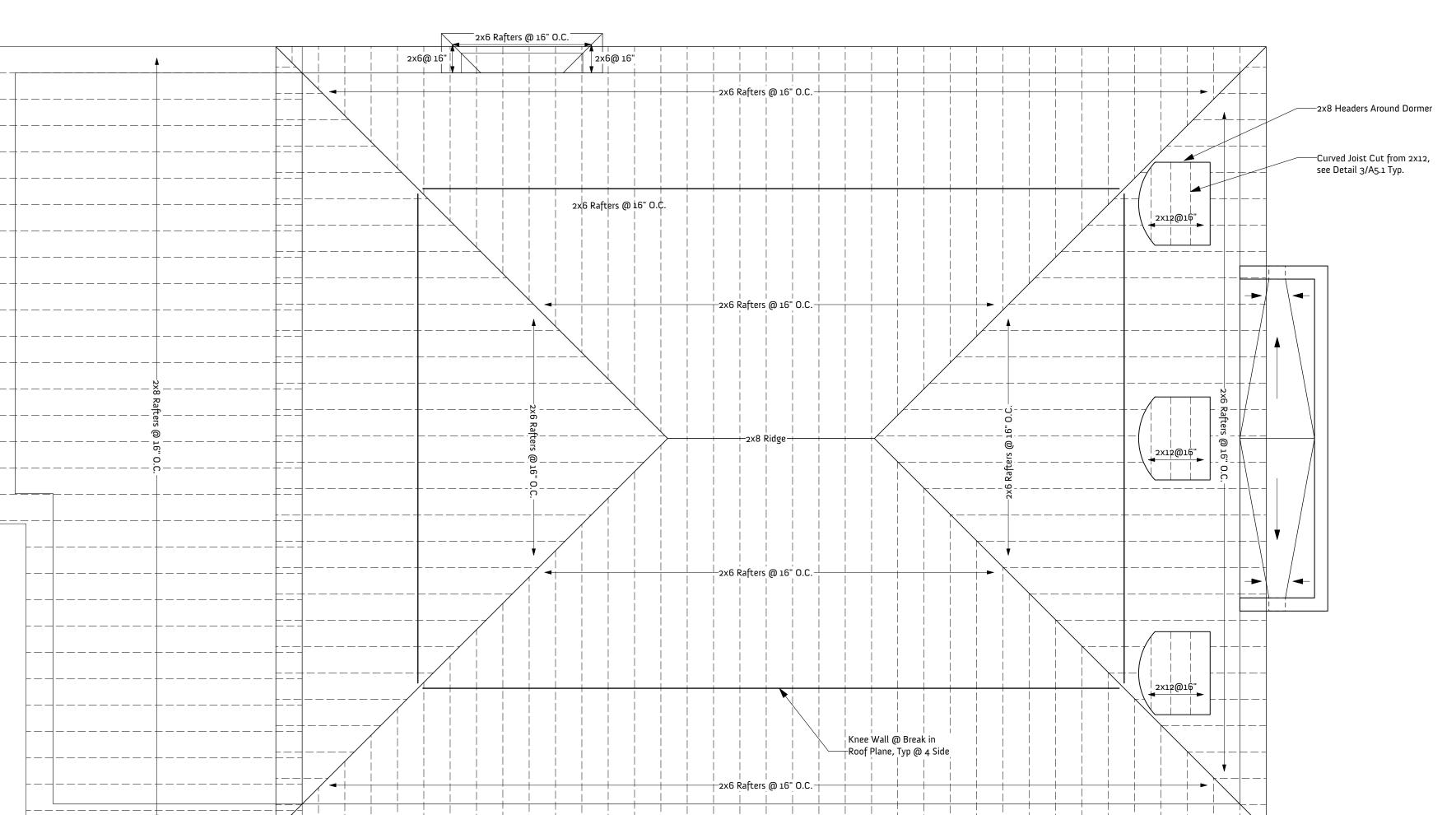








2 Typical Knee Wall Section
A3.1 Scale: 1" = 1'-0"





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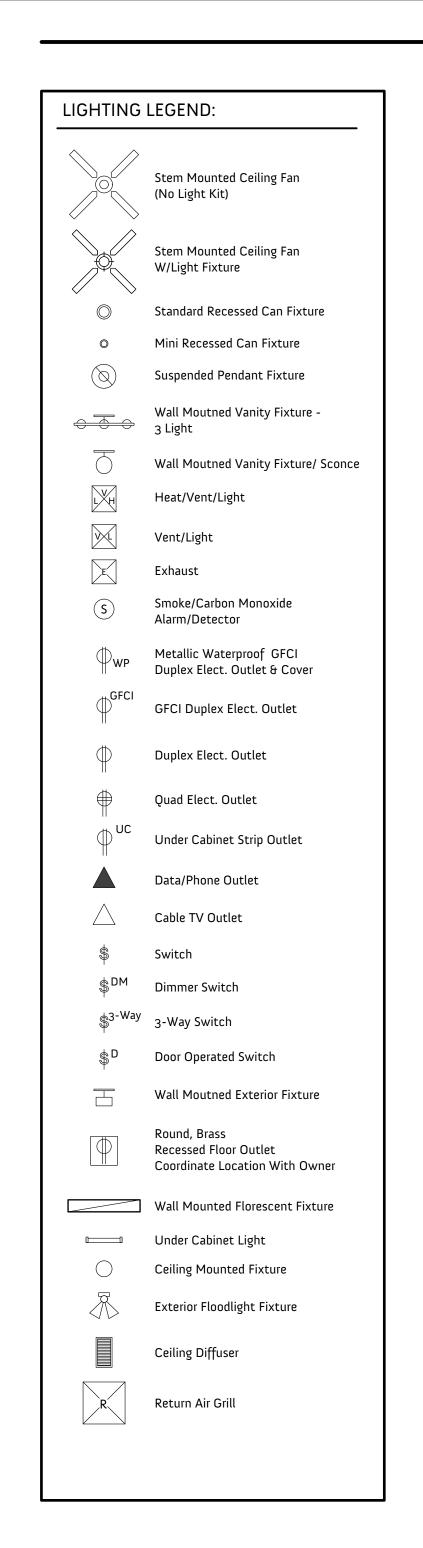
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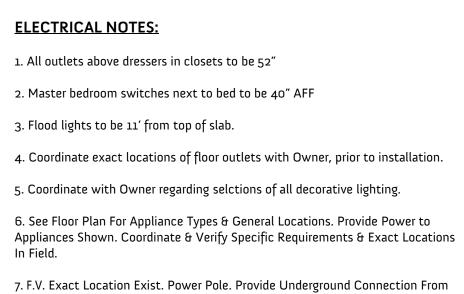
Date	To	Use / Restriction
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06.17.22	Owner	DD Progress
07.07.22	Owner	Design Development
07.22.22	Owner	Construction Docs.
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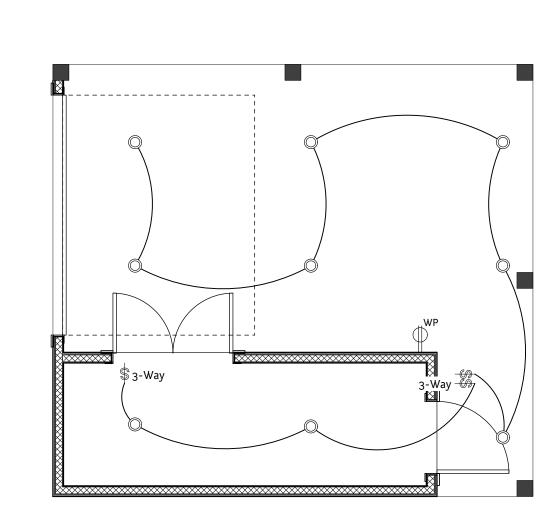
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Residence to Pole.







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Single Family Residen 6544 Vicksburg St.

Date To Use / Restriction

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06.17.22 Owner DD Progress

07.07.22 Owner Design Development

07.22.22 Owner Construction Docs.

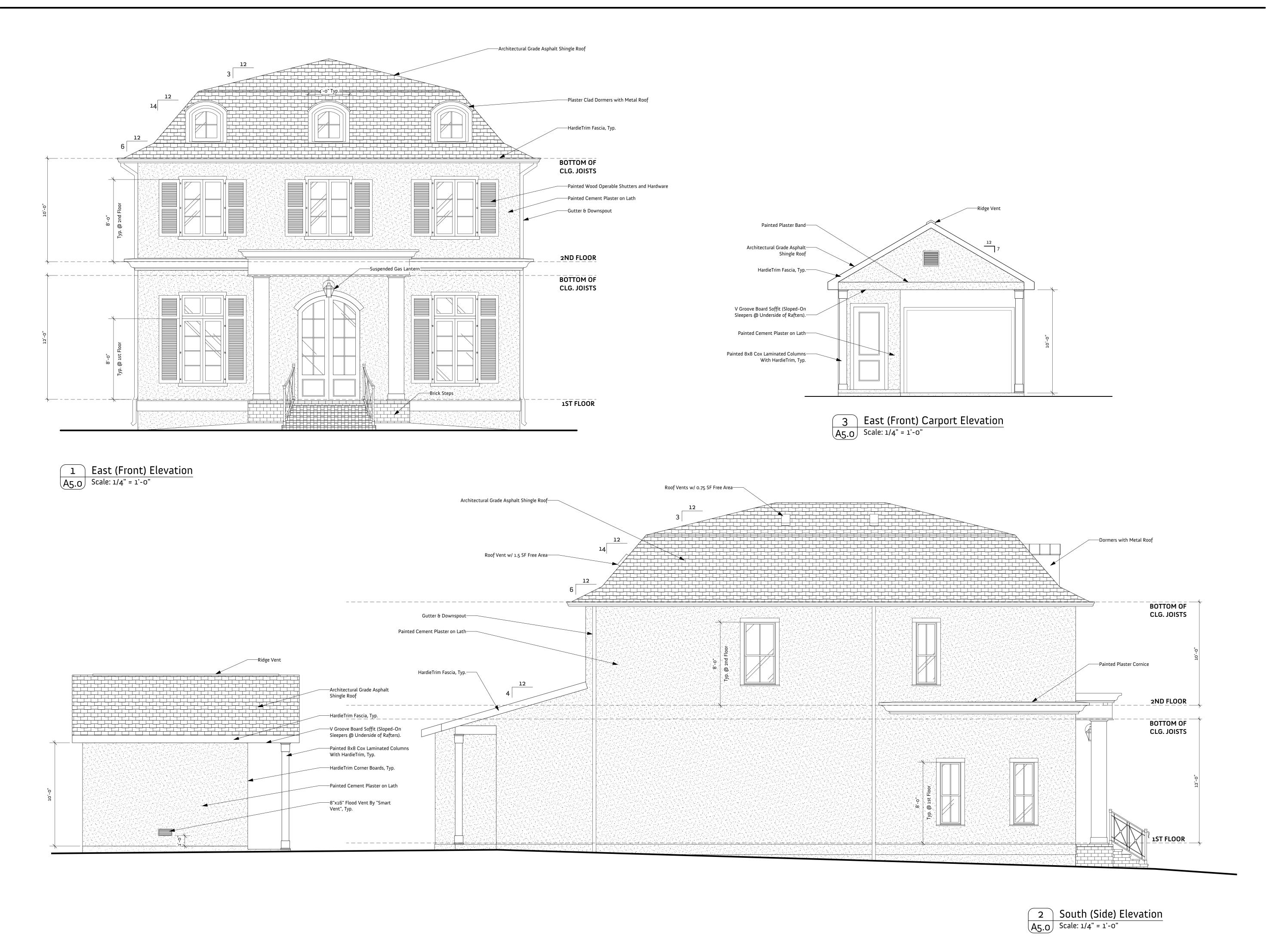
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Release



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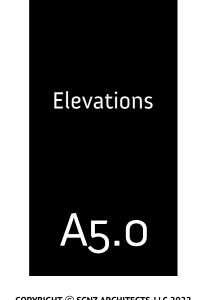
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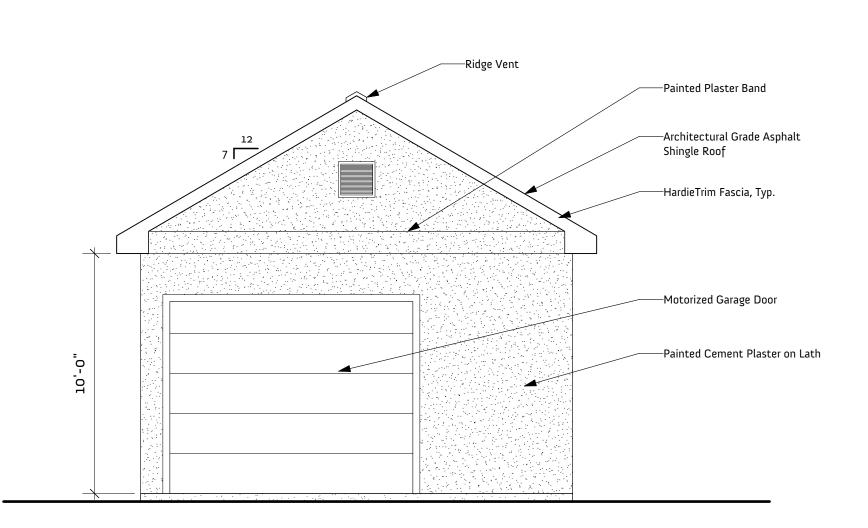
Issued Use / Restriction

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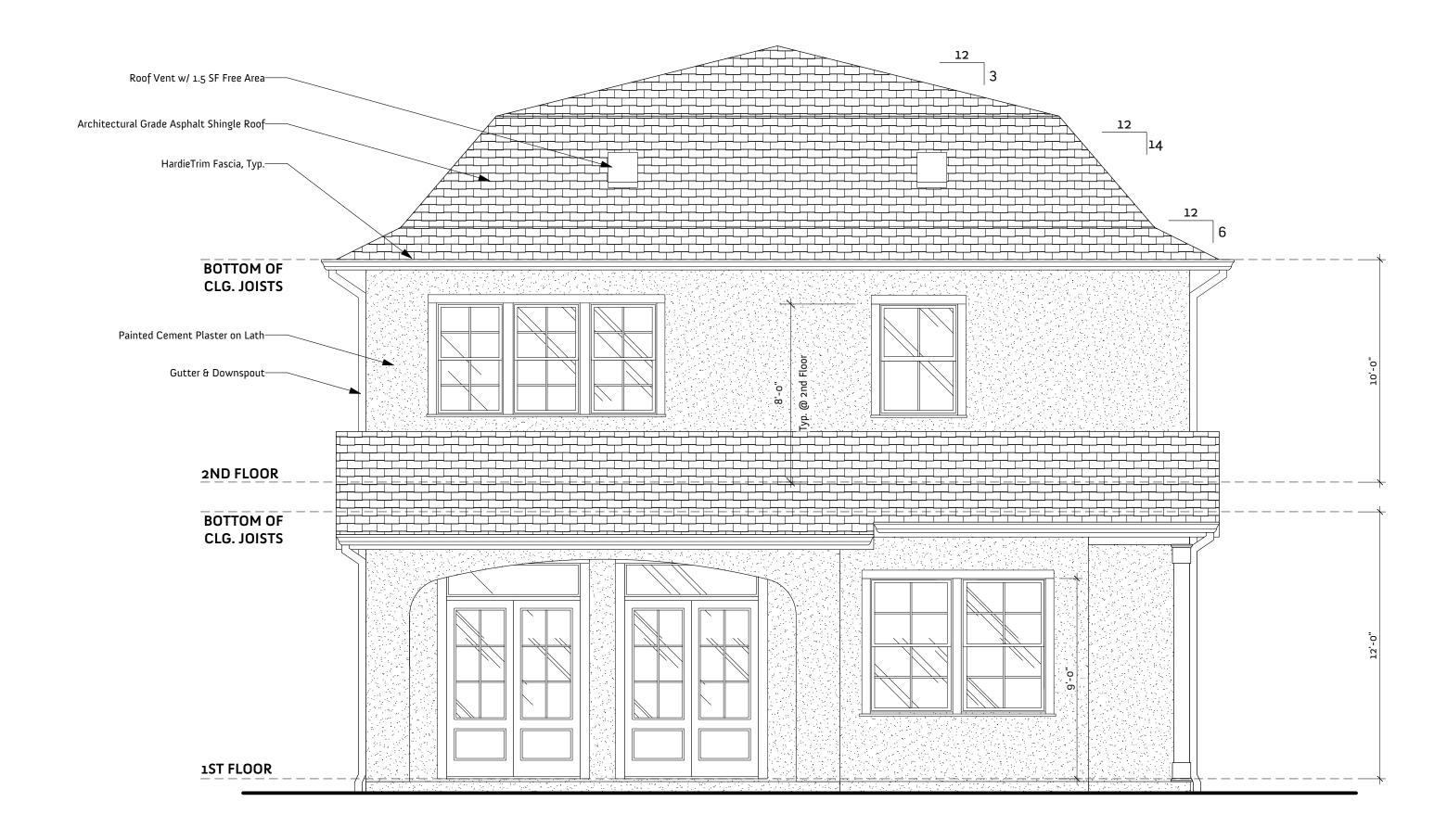


Project No. 2216 Drawn By JPL





3 West (Rear) Carport Elevation A5.1 Scale: 1/4" = 1'-0"



West (Rear) Elevation A5.1 Scale: 1/4" = 1'-0" ----Roof Vents w/ 0.75 SF Free Area —Architectural Grade Asphalt Shingle Roof Dormers with Metal Roof -Roof Vent w/ 1.5 SF Free Area BOTTOM OF CLG. JOISTS —Gutter & Downspout —Painted Cement Plaster on Lath Painted Plaster Cornice—— —HardieTrim Fascia, Typ. —Architectural Grade Asphalt Standing Seam Metal Roof Shingle Roof 2ND FLOOR -HardieTrim Fascia, Typ. BOTTOM OF CLG. JOISTS -V Groove Board Soffit (Sloped-On Sleepers @ Underside of Rafters). —Painted 8x8 Cox Laminated Columns With HardieTrim, Typ. —Painted Cement Plaster on Lath —8"x16" Flood Vent By "Smart

North(Side) Elevation
A5.1 Scale: 1/4" = 1'-0"

ARCHITECTS

2134 Magazine St. Suite 200

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> Single Family Residence 6544 Vicksburg St.

Date To Use / Restriction

05.20.22 Owner Schematic Design

06.17.22 Owner DD Progress

07.07.22 Owner Design Development

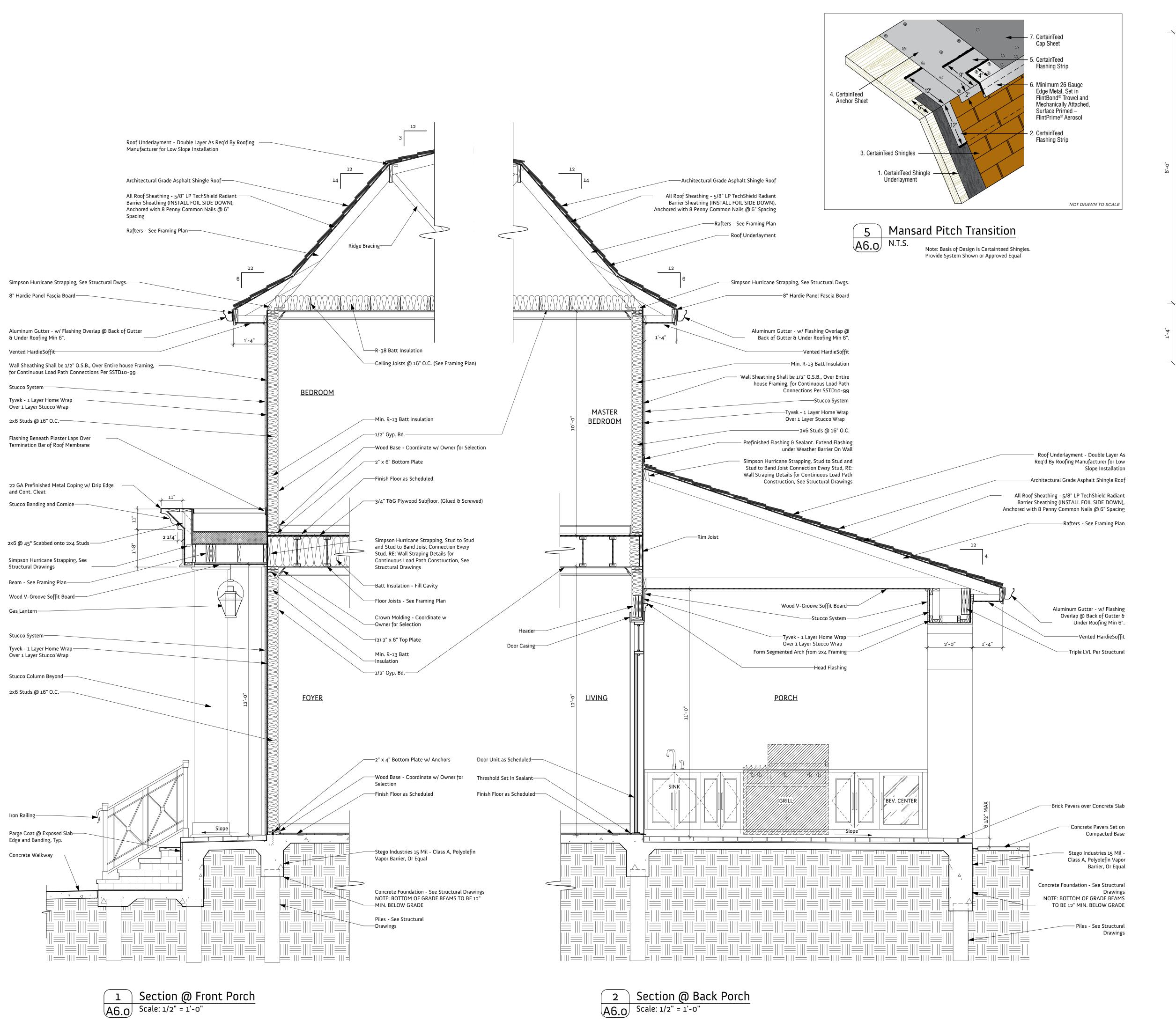
07.22.22 Owner Construction Docs.

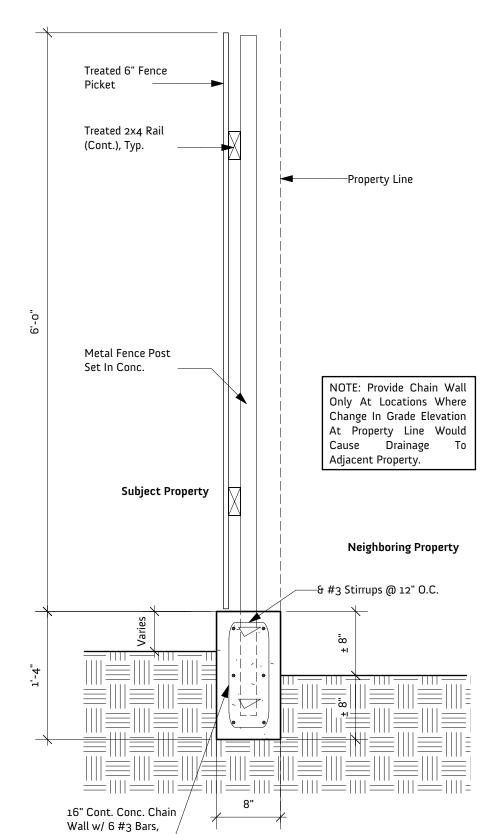
Progress Release



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6 Chain Wall Fence Detail A6.0 Scale: 1" = 1'-0"

## Issued

Release

Date	To	Use / Restriction
05.20.22	Owner	Schematic Design
06.17.22	Owner	DD Progress
07.07.22	Owner	Design Development
07.22.22	Owner	Construction Docs.
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Signal Signal



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Elevations
A6.0

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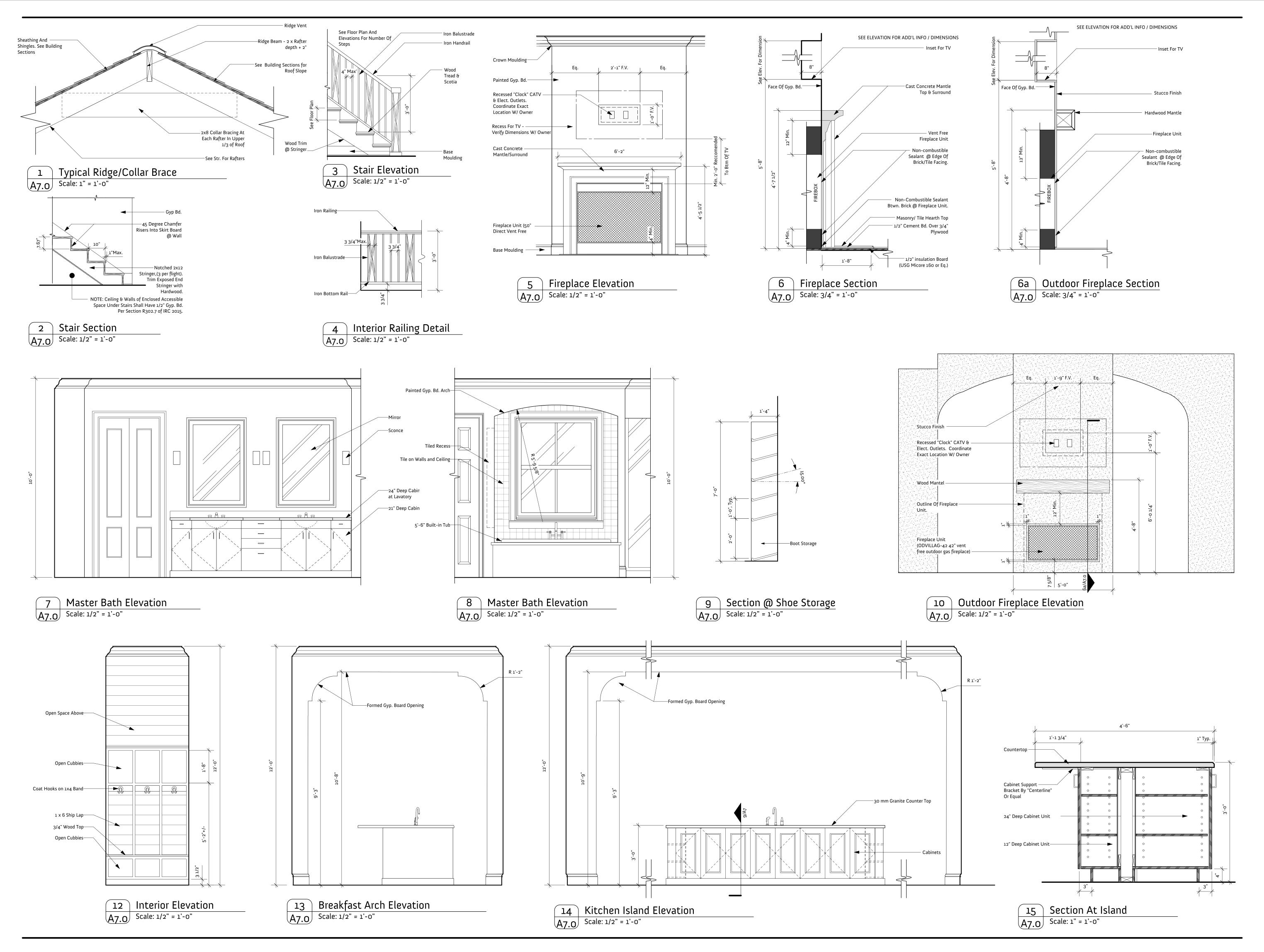
HURRICANE COMPLIANCE:

• Residence Is Designed To Withstand 130 M.P.H. Winds In Accordance W/ The Minimum Design Loads For Buildings & Other Structures (ASCE-7).

• In Accordance With The International Building Code 2015, 1609, Building Shall Be Anchored Against Overturning, Uplift & Sliding. Structural Members & Systems & Components & Cladding In Building Shall Be Anchored To Resist Wind-induced Overturning, Uplifting & Sliding And To Provide Continuous Load Paths For These Forces To The Foundation. See Structural Drawings.

• Windows Shall Comply With 2015 International Residential Code R301.2.1.2 Windows Shall Be Tested For 130 M.P.H. Wind Speed Or Windows Shall Have Glazed Openings Protected From Windborne Debris. Exception: 1/2" Plywood Panels Shall Be Provided For All Window Openings W/Attachment Hardware.

• Window Panels and Fasteners for Wind Borne Debris Protection Shall Be Provided in Accordance with R301.2.1.2 IRC 2015.

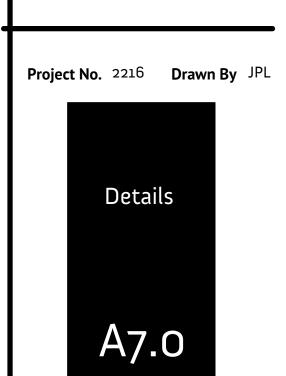


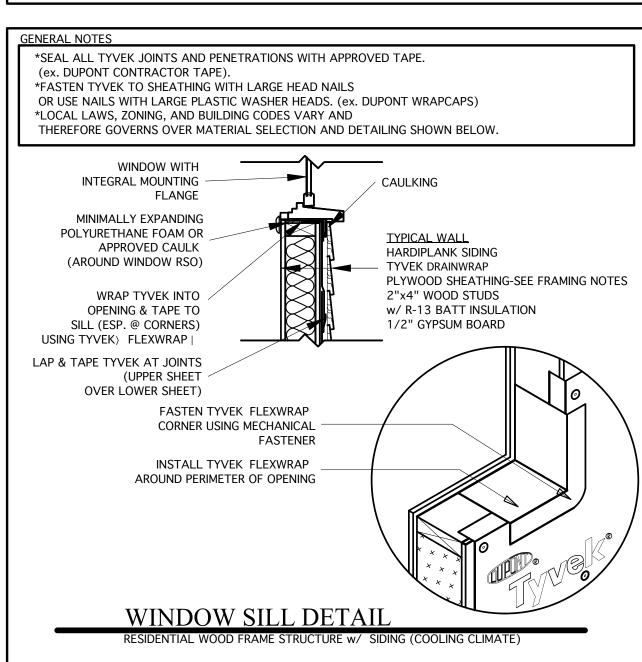


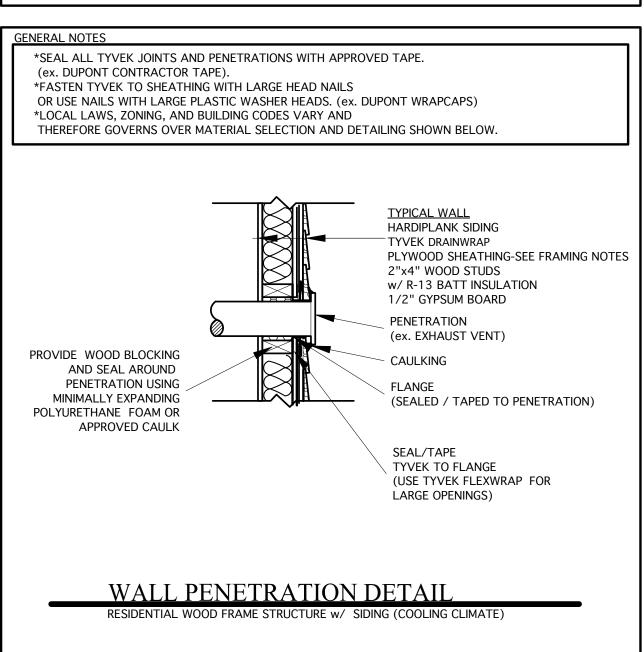
Single Family Residence 6544 Vicksburg St.

Date	То	Use / Restriction
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		1









Tyvek Flashing Details

A8.0 N.T.S.

### Integral Flanged Window AFTER Water-Resistive Barrier (WRB) is Installed

Method applies to following products:

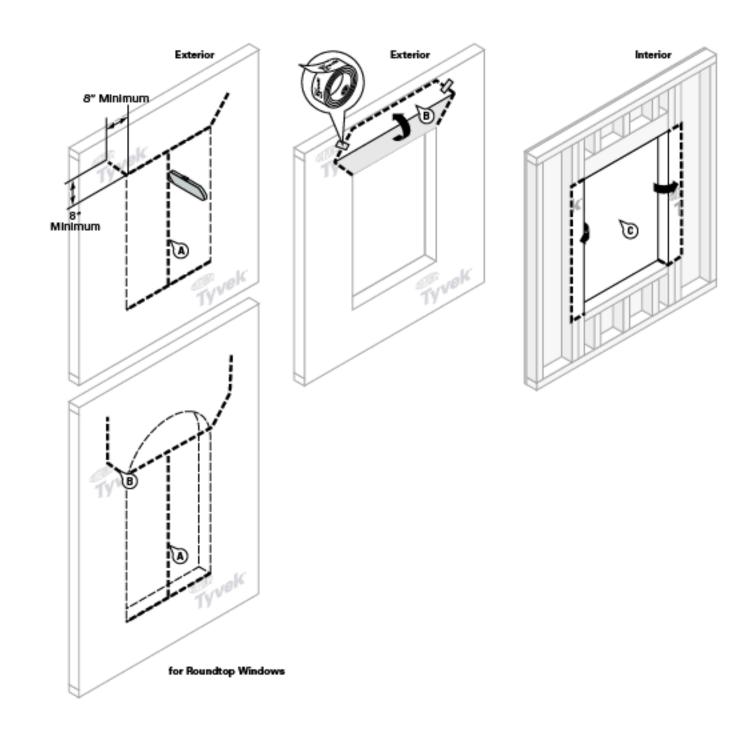
DuPont StraightFlash

DuPont" FlexWrap"

#### STEP 1

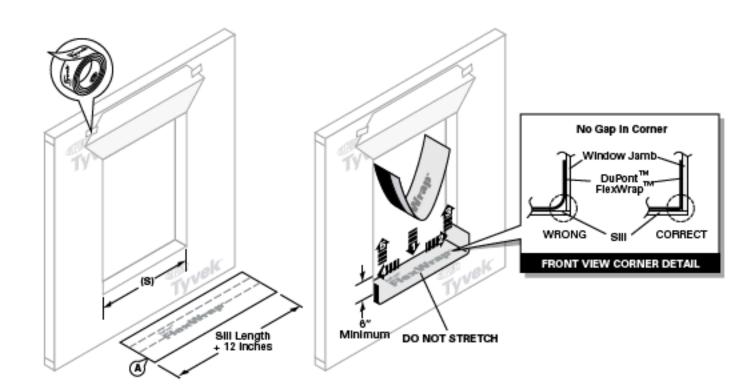
Prepare WRB for window installation:

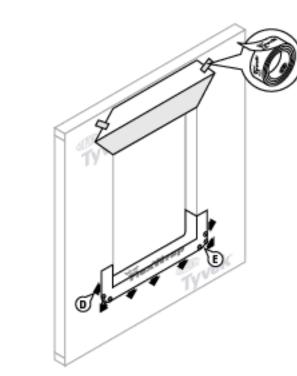
- A. Make an "I-Cut" (Standard I-Cut) in the WRB (modified I-Cut is also accepted). For an "I-Cut", begin with a horizontal cut across the bottom and the top of the window frame (for round top windows, the cut should begin above the mull joint). From the center, cut straight down to the sill.
- B. Cut two 45 degree slits a minimum of 8" from the corner of the header to create a flap above the rough opening to expose sheathing or framing members to allow head flashing installation (see step 4). Flip head flap up (B) and temporarily secure with DuPont "Tyvek" Tape. Some windows and flashing widths may require longer slits.
- C. Fold side flaps into rough opening, cut excess flaps, and secure.



#### STEP 2

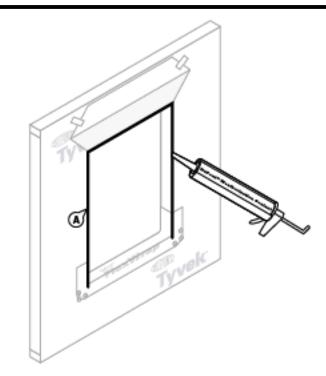
- A. Cut DuPont FlexWrap at least 12" longer than width of rough opening sill (S).
- B. Remove first piece of release paper, cover horizontal sill by aligning inside edge of sill, and adhere into rough opening along sill and up jambs (min. 6" on each side).
- C. Remove second release paper.
- D. Fan out DuPont™ FlexWrap™ at bottom corners onto face of wall. Coverage of DuPont™ FlexWrap™ should be 2" to 3" onto the face of the wall.
- E. SECURE EDGES OF DUPONT" FLEXWRAP" WITH DUPONT RECOMMENDED FASTENERS along the bottom outer edge of the DuPont" FlexWrap" at fanned corners.





#### STEP 3

A. Apply continuous bead of DuPont™ Weatherization
Sealant at the window head and jambs to wall or back
side of window mounting flange. DO NOT APPLY
CONTINUOUS SEALANT BEAD ACROSS BOTTOM
SILL to allow for drainage.

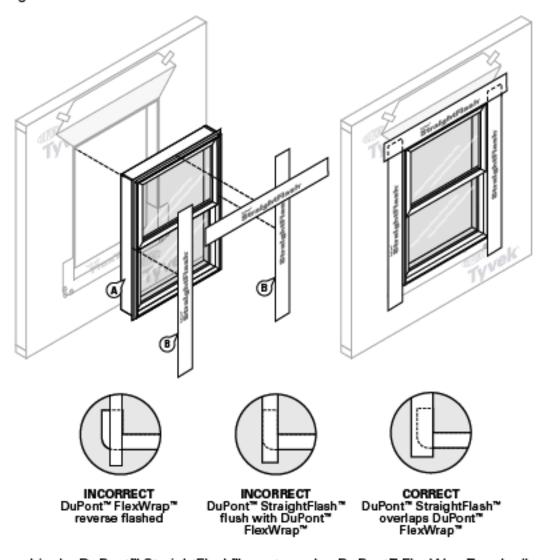


#### FOR RECTANGULAR WINDOWS

#### STEP 4

A. Install window according to manufacturer's instructions.

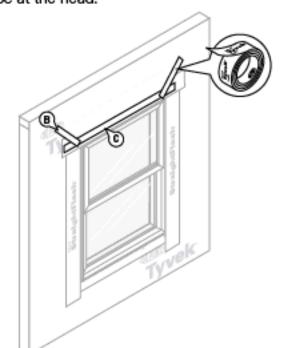
- B. Cut two pieces of DuPont™ StraightFlash™ or DuPont™ FlexWrap™ for jamb flashing extending 1" above window head flange and below bottom edge of sill flashing. Remove release paper and press tightly along sides of window frame.
- C. Cut a piece of DuPont™ StraightFlash™ or DuPont™ FlexWrap™ for head flashing, which extends beyond outer edges of jamb flashings. Remove release paper and install completely covering mounting flange and adhering to exposed sheathing or framing members.



**NOTE**: Do not reverse shingle. DuPont™ StraightFlash™ must overlap DuPont™ FlexWrap™ and adhere to the substrate.

## A. Flip down upper flap of WRB so it lays flat across head flashing.

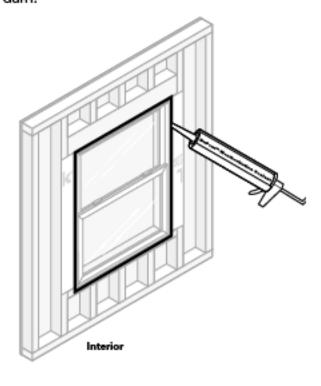
- B. Tape down diagonal seams of WRB.
- C. Tape seams as shown. DO NOT TAPE at bottom of window. At the head, continuous tape seams as shown with DuPont™ Tyvek® Tape; if an air barrier is not required or if additional drainage is desired, then skip-tape at the head.



#### STEP 6

Final Step

A. Tool sealant around the window opening at the interior, using DuPont™ Weatherization Sealant or DuPont Recommended Low Expansion Foam (and backer rod as necessary). Sealant and backer rod will also serve as a back dam.



2 Tyvek Window Flashing Installation A8.0 N.T.S.



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		1
		1

Progress

Release



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## GENERAL STRUCTURAL NOTES:

- UNLESS NOTED OTHERWISE ON THE DRAWINGS, THE FABRICATION, TESTING, AND CONSTRUCTION SHALL BE IN STRICT ACCORDANCE WITH THE FOLLOWING NOTES. SHOULD CODES OR STANDARDS CONFLICT WITH THE DRAWINGS AND SPECIFICATIONS, THE MORE STRINGENT REQUIREMENT SHALL GOVERN. REFER TO THE PROJECT SPECIFICATIONS FOR ADDITIONAL INFORMATION AND REQUIREMENTS.
- 2. FOR THE FOLLOWING REFERENCE CODES AND STANDARDS, ONLY THE LATEST EDITION IS APPLICABLE, UNLESS OTHERWISE INDICATED:
  - (A) AMERICAN CONCRETE INSTITUTE (ACI)
  - (B) AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC)
  - (C) AMERICAN IRON AND STEEL INSTITUTE (AISI)
  - (D) AMERICAN STANDARD FOR TESTING AND MATERIALS (ASTM)
  - (E) AMERICAN WELDING SOCIETY (AWS)
  - (F) RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (RCSC)
  - (G) STEEL STRUCTURES PAINTING COUNCIL (SSPC)
  - (H) OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION (OSHA)
- SPECIFIED MATERIALS INCLUDING GROUTS, SEALANTS, ANCHORAGE, MECHANICAL DEVICES, ETC. SHALL BE IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND REQUIREMENTS SET OUT IN THE SPECIFICATIONS.
- STRUCTURAL DRAWINGS SHALL BE USED AND INTERPRETED IN CONJUNCTION AND COORDINATION WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, SHOP DRAWINGS, AND SPECIFICATIONS.
- 5. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ELEVATIONS SET OUT IN THE ARCHITECT'S DRAWINGS BEFORE COMMENCING WORK.
- CONTRACTOR SHALL VERIFY ALL CAMBER, DEPRESSIONS, SLOPES, OPENINGS, PENETRATIONS, ETC. THROUGH OR WITHIN STRUCTURAL ELEMENTS. ANY STRUCTURAL ELEMENTS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE
- CONTRACTOR SHALL VERIFY THE LOCATION OF ALL EXISTING UTILITIES BEFORE BEGINNING ANY WORK. ANY INTERFERENCE OR CONFLICT SHALL BE BROUGHT IMMEDIATELY TO THE ATTENTION OF THE ENGINEER OF RECORD.
- CONTRACTOR SHALL BE RESPONSIBLE FOR ALL FINAL DIMENSIONS AND FIT-UP OF THE STRUCTURE, INCLUDING BUT NOT LIMITED TO, VERIFYING ALL EXISTING CONDITIONS AND DIMENSIONS BEFORE COMMENCING WORK AND ALL AS-BUILT CONDITIONS AS THE WORK
- CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE DESIGN, ERECTION, PLACEMENT, MAINTENANCE, DURATION AND REMOVAL OF ANY SHORING, RE-SHORING, BACK-SHORING, BRACING, TIE-BACKS, OR OTHER TEMPORARY SUPPORT STRUCTURES REQUIRED TO SUPPORT ANY PART OF THE NEW OR EXISTING CONSTRUCTION OR SURROUNDING IMPROVEMENTS DURING THE ENTIRE CONSTRUCTION PROCESS TO ENSURE
- 10. ALL WORK AREAS SHALL BE KEPT NEAT, CLEAN, AND SAFE AT ALL TIMES BY THE CONTRACTOR. TRASH AND DEMOLISHED MATERIALS SHALL NOT BE ALLOWED TO ACCUMULATE AT THE SITE DURING EXECUTION OF WORK. CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVAL OF ALL DEBRIS. ALL DEBRIS SHALL BE PROPERLY AND LEGALLY DISPOSED OF. ALL ASPECTS OF JOB SITE SAFETY ARE COMPLETELY THE RESPONSIBILITY OF THE CONTRACTOR.
- STEEL FRAMES ARE "NON-SELF SUPPORTING". ADEQUATE TEMPORARY SUPPORT SHALL BE PROVIDED BY THE CONTRACTOR UNTIL REQUIRED CONNECTIONS OR ELEMENTS ARE INSTALLED AND COMPLETED.
- 12. DETAILS SHOWN ON DRAWINGS ARE TO BE CONSIDERED TYPICAL FOR ALL SIMILAR CONDITIONS.
- 13. CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR AND HAVE CONTROL AND CHARGE OF THE MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, AND FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK.

## FOUNDATION NOTES:

THE SAFETY AND STABILITY OF THE STRUCTURE

- 1. FOUNDATION DESIGN IS BASED ON ORLEANS PARISH PILE LOAD CAPACITY MAPS AREA GM-10.
- 2. PILE TYPE:
  - PILING SHALL BE ONE PIECE TREATED TIMBER PILING (CCA 0.80) SHOWN THUS AND SHALL CONFORM TO ANSI 05.1 CLASS 5 AND HAVE A MINIMUM TIP DIAMETER OF 6 INCHES AND A MINIMUM BUTT DIAMETER OF 8 INCHES MEASURED 3 FEET FROM THE END OF THE PILE. THE TIPS OF ALL PILES SHALL BE DRIVEN TO AN ELEVATION OF 30 FEET BELOW EXISTING GRADE.
    - (A) DESIGN LOAD = 8 TONS
    - (B) CUT-OFF TREATMENT: BRUSH TREAT TOP OF PILE WITH COPPER NAPTHENATE CONFORMING TO A.W.P.A. SPECIFICATION M4
    - (C) HAMMER:
      - SINGLE ACTING AIR HAMMER DELIVERING 7,250 FT. LBS. OF ENERGY PER BLOW, VIBRATORY OR COMPACTION HAMMER NOT PERMITTED
    - (D) DRIVE TO A REFUSAL OF 12 BLOWS PER FOOT MAX
    - (E) PILE DRIVER TO MONITOR PILE INSTALLATION FOR VIBRATION AND PROVIDE VIBRATION REPORTS TO ENGINEER OF
- 3. UNLESS SHOWN OTHERWISE, GRADE BEAMS SHALL BE CENTERED ON COLUMNS AND WALLS.
- 4. GRADE BEAMS MAY BE EARTH FORMED PROVIDED DIMENSIONAL TOLERANCES LISTED IN THE APPLICABLE ACI CODES ARE ADHERED
- PLACE 6 MIL. WATERPROOF MEMBRANE BENEATH ALL INTERIOR SLABS AND BEAMS ON GRADE. LAP 12" TO ACCOMMODATE CONCRETE POURING DIRECTION.

## CONCRETE NOTES

- 1. APPLICABLE CODES OR STANDARDS:
  - ALL DESIGN, FABRICATION, TESTING, AND ERECTION SHALL BE IN ACCORDANCE WITH THE FOLLOWING CODES AND STANDARDS:
  - (A) ACI 117 SPECIFICATION FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS
  - (B) ACI 301 SPECIFICATIONS FOR STRUCTURAL CONCRETE
  - (C) ACI 304 RECOMMENDED PRACTICE FOR MEASURING, MIXING, TRANSPORTING, AND PLACING CONCRETE
  - (D) ACI 308 RECOMMENDED PRACTICE FOR CURING CONCRETE
  - (E) ACI 315 AND 315R DETAILS AND DETAILING OF CONCRETE REINFORCEMENT
  - (F) ACI 316 RECOMMENDED PRACTICE FOR CONSTRUCTION OF CONCRETE PAVEMENTS AND CONCRETE BASES
  - (G) ACI 318 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
  - (H) ACI 336 SUGGESTED DESIGN AND CONSTRUCTION PROCEDURES FOR PIER FOUNDATIONS
  - (I) ACI 347 RECOMMENDED PRACTICE FOR CONCRETE FORM WORK
  - (J) ASTM STANDARDS FOR THE MATERIALS LISTED.

## CONCRETE NOTES (CONT.):

#### 2. MATERIALS:

MATERIALS SHALL MEET THE SPECIFICATIONS LISTED IN THIS SECTION (UNLESS NOTED OTHERWISE):

- (A) CONCRETE SHALL A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI AT 28 DAYS.
- (B) CONCRETE SHALL BE NORMAL WEIGHT (APPROXIMATELY 150 LBS. PER CUBIC FT.)
- (C) PORTLAND CEMENT SHALL MEET ASTM C150 TYPE II.
- (D) AGGREGATE FOR NORMAL WEIGHT CONCRETE SHALL MEET ASTM C33.
- (E) REINFORCING STEEL SHALL MEET ASTM A615 GRADE 60.
- (F) WELDED WIRE FABRIC (WWF) SHALL MEET ASTM A185.
- (G) STEEL PLAIN WIRE SHALL MEET ASTM A82.

CONCRETE SLUMPS SHALL MEET THE SPECIFICATIONS LISTED IN THIS SECTION (UNLESS NOTED OTHERWISE):

- (A) CONCRETE WITHOUT WATER-REDUCING ADMIXTURES OR PRIOR TO THEIR ADDITIONS SHALL HAVE A MAXIMUM SLUMP OF
- (B) CONCRETE WITH LOW TO MODERATE RANGE WATER-REDUCING ADMIXTURES SHALL HAVE A MAXIMUM SLUMP OF 6
- (C) CONCRETE WITH HIGH RANGE WATER-REDUCING ADMIXTURES SHALL HAVE A MAXIMUM SLUMP OF 8 INCHES.
- EXPOSED EDGE CONDITIONS:
  - (A) EXPOSED EDGES OF CONCRETE ABOVE GRADE SHALL BE CHAMFERED 3/4" AT 45 DEGREES (AS SHOWN ON SECTIONS IF REQUIRED).

#### 5. BONDING:

BONDING SHALL MEET THE SPECIFICATIONS LISTED IN THIS SECTION (UNLESS NOTED OTHERWISE):

- (A) CONSTRUCTION JOINTS BETWEEN NEW AND HARDENED CONCRETE SHALL BE CLEAN, FREE OF LAITANCE, AND INTENTIONALLY ROUGHENED TO A FULL AMPLITUDE OF 1/4".
- (B) FOR INSTALLATION OF DOWELS IN HARDENED CONCRETE, CONTRACTOR SHALL DRILL AND EPOXY WITH HILTI HY-HIT 200 OR APPROVED EQUAL.
- (C) FOR INSTALLATION OF DOWELS IN BRICK MASONRY, CONTRACTOR SHALL DRILL AND EPOXY WITH HILTI HY-HIT 270 OR APPROVED EQUAL.
- 6. CONCRETE PROTECTION FOR REINFORCEMENT:

CONTRACTOR SHALL PROVIDE PROTECTIVE COVER FOR REINFORCING LISTED IN THIS SECTION (UNLESS NOTED OTHERWISE):

- (A) 3" FOR CONCRETE GRADE BEAMS AND FOOTINGS DEPOSITED DIRECTLY AGAINST THE GROUND.
- (B) 2" FOR FORMED CONCRETE EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND.
- (C) 1" FOR CONCRETE SLABS AND WALLS NOT EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND
- (D) 1 1/2" FOR CONCRETE BEAMS, GIRDERS, AND COLUMNS NOT EXPOSED TO WEATHER OR IN CONTACT WITH THE

#### PLACEMENT:

PLACEMENT SHALL MEET THE SPECIFICATIONS LISTED IN THIS SECTION (UNLESS NOTED OTHERWISE):

- (A) BARS SHALL BE SECURELY SUPPORTED TO PREVENT BOTH VERTICAL AND HORIZONTAL MOVEMENT DURING CONCRETE
- (B) REINFORCING BARS OR FABRIC ON GRADE SHALL BE CHAIRED WITH 3000 PSI CONCRETE BRICKETTES SPACED ADEQUATELY TO SUPPORT THE REINFORCING, BUT NOT GREATER THAN 3'-0" O.C. EACH WAY. AT RAISED FLOORS USE METAL CHAIRS.
- (C) PROVIDE A 90 DEGREE HOOK ON ALL TOP REINFORCING IN ALL BEAMS AT DISCONTINUOUS ENDS AND LAP SPLICE 30 BAR DIAMETERS AT MID-SPAN.
- (D) CONTINUOUS BOTTOM BARS SHOULD BE LAP SPLICED 6" AT CENTER OF SUPPORT.
- (E) LAP ALL WELDED WIRE FABRIC ONE WIRE SPACING PLUS 6 INCHES.
- (F) COLUMN VERTICAL REINFORCING SHALL HAVE STANDARD HOOKS AT THE TOP OF THE UPPERMOST SECTION OF EACH
- (G) PROVIDE CORNER BARS AT EACH OUTSIDE CORNER FOR EACH HORIZONTAL BAR IN WALLS AND BEAMS. HOOK INSIDE BAR IN WALLS AT ENDS.
- (H) PLACEMENT OF SLEEVES. HOLES. OR OPENINGS THROUGH BEAMS. FOOTINGS. PILE CAPS. SLABS. ETC. IS NOT PERMITTED WITHOUT ENGINEER OF RECORD'S APPROVAL
- WHERE POSSIBLE, EXISTING REINFORCEMENT SHALL NOT BE CUT, BENT, OR DAMAGED. WHENEVER REINFORCEMENT IS CUT, DAMAGED OR BENT, IT SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD. REINFORCEMENT SHALL BE REPAIRED OR REPLACED AS DIRECTED.

#### 8. SPLICES:

REINFORCEMENT STEEL SPLICES SHALL MEET THE SPECIFICATIONS LISTED IN THIS SECTION (UNLESS NOTED OTHERWISE):

- (A) REINFORCING BARS SHALL BE SPLICED WITH CLASS "B" LAP SPLICES.
- (B) PROVIDE REQUIRED LAP LENGTHS FOR CORNER BARS, TEMPERATURE BARS IN SLAB, INTERMEDIATE HORIZONTAL BARS IN WALLS AND BEAMS, ETC.
- 9. EXPANSION JOINTS AND JOINT SEALERS:

EXPANSION JOINTS SHALL MEET THE SPECIFICATIONS LISTED IN THIS SECTION (UNLESS NOTED OTHERWISE):

- (A) EXPANSION JOINT MATERIAL SHALL BE 1/2" THICK SEAL—TIGHT ASPHALT EXPANSION JOINT FILLER OR APPROVED
- (B) EXPANSION JOINTS SHALL SEPARATE PAVING FROM FOUNDATION GRADE BEAMS, FOOTINGS, ETC. AS SHOWN ON

## CONCRETE NOTES (CONT.):

#### 10. EMBEDMENTS:

CONDUITS, PIPES, ETC. EMBEDDED IN CONCRETE SHALL MEET THE SPECIFICATIONS LISTED IN THIS SECTION (UNLESS NOTED OTHERWISE):

- (A) CONTRACTOR SHALL SUBMIT FOR APPROVAL A DIAGRAM DEPICTING ALL CONDUITS, PIPES, OR SLEEVES EMBEDDED IN
- (B) CONTRACTOR SHALL FOLLOW ALL REGULATIONS OUTLINED IN THE APPLICABLE ACI CODES FOR EMBEDDING CONDUITS,
- (C) CONDUITS, PIPES, AND SLEEVES OF ANY MATERIAL NOT HARMFUL TO CONCRETE SHALL BE PERMITTED TO BE
- (D) IT WILL NOT BE PERMITTED TO CUT, BEND, OR DISPLACE THE REINFORCING STEEL FROM ITS PROPER LOCATION TO INSTALL CONDUITS. PIPES. ETC. WITHOUT THE ENGINEER OF RECORD'S APPROVAL.
- (E) CONDUITS, PIPES, AND SLEEVES PASSING THROUGH A SLAB, BEAM, OR WALL SHALL NOT SIGNIFICANTLY IMPAIR THE STRENGTH OF CONSTRUCTION.
- (F) OUTSIDE DIMENSIONS FOR SINGLE CONDUITS AND PIPES OR INTERSECTING CONDUITS AND PIPES SHALL NOT OCCUPY MORE THAN AN 1/3 THE OVERALL THICKNESS OF SLAB, BEAM, OR WALL IN WHICH THEY ARE EMBEDDED. ANY CONDUIT OR PIPE LARGER SHALL BE LOCATED BELOW THE RESPECTIVE SLAB OR BEAM.
- (G) CONDUITS, PIPES, ETC. SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS OR WIDTHS ON CENTER.

EMBEDDED IN CONCRETE WITH THE ENGINEER OF RECORD'S APPROVAL

11. DRILLING HOLES OR CORING HOLES IN EXISTING CONCRETE:

DRILLING OR CORING HOLES IN EXISTING CONCRETE SHALL MEET THE SPECIFICATIONS LISTED IN THIS SECTION (UNLESS NOTED OTHERWISE):

- (A) PRIOR TO DRILLING OR CORING HOLES, THE CONTRACTOR SHALL LOCATE ALL EXISTING REINFORCING STEEL, POST-TENSIONING, CONDUIT, PIPING, ETC. THROUGH NON-DESTRUCTIVE TESTING SUCH AS WITH AN X-RAY, RADAR,
- (B) CONTRACTOR SHALL MARK THE LOCATION OF ALL REINFORCING STEEL, POST-TENSIONING, CONDUIT, PIPING, AND OTHER EXISTING INTERFERENCES ON THE SURFACE OF THE CONCRETE.
- (C) CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD FOR ALL CONFLICTS BETWEEN NEW HOLES AND EXISTING REINFORCING, POST-TENSIONING, CONDUIT, PIPING, ETC.
- (D) CONTRACTOR SHALL DRILL SMALL PILOT HOLES AT NEW HOLE LOCATIONS TO VERIFY NO CONFLICTS EXIST. IF NO CONFLICTS EXIST, COMPLETE THE INSTALLATION. IN THE CASE OF STEEL TO BE FASTENED TO THE EXISTING CONCRETE WITH MULTIPLE ANCHORS, FABRICATE FROM A FIELD TEMPLATE, THE STEEL TO BE FASTENED TO THE CONCRETE BY THE ANCHORS AND COMPLETE THE INSTALLATION.
- (E) CONTRACTOR SHALL EXERCISE CARE WHEN INSTALLING NEW HOLES TO PREVENT "NICKING" OR CUTTING EXISTING REINFORCING STEEL, POST-TENSIONING, CONDUIT, PIPING, ETC.

## 12. QUALITY CONTROL TESTING DURING CONSTRUCTION

- (A) GENERAL: EMPLOY A TESTING AGENCY TO PERFORM TESTS AND TO SUBMIT TEST REPORTS.
- (B) SAMPLING AND TESTING FOR QUALITY CONTROL DURING CONCRETE PLACEMENT SHALL INCLUDE THE FOLLOWING, AS DIRECTED BY
  - 1. SAMPLING FRESH CONCRETE: ASTM C 172, EXCEPT MODIFIED FOR SLUMP TO COMPLY WITH ASTM C 94

ASTM C 143; ONE TEST AT POINT OF DISCHARGE FOR EACH DAY'S POUR OF EACH TYPE OF CONCRETE, ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY SEEMS TO HAVE CHANGED.

ASTM C 173, VOLUMETRIC METHOD FOR LIGHTWEIGHT OR NORMAL WEIGHT CONCRETE; ASTM C 231, PRESSURE METHOD FOR NORMAL WEIGHT CONCRETE; ONE FOR EACH DAY'S POUR OF EACH TYPE OF AIR-ENTRAINED CONCRETE.

(27 DEG C) AND ABOVE, AND ONE TEST FOR EACH SET OF COMPRESSIVE—STRENGTH SPECIMENS.

ASTM C 1064; ONE TEST HOURLY WHEN AIR TEMPERATURE IS 40 DEG F (4 DEG C) AND BELOW, WHEN 80 DEG F

COMPRESSION TEST SPECIMEN: ASTM C 31, ONE SET OF FOUR STANDARD CYLINDERS FOR EACH COMPRESSIVE-STRENGTH TEST, UNLESS OTHERWISE DIRECTED. MOLD AND STORE CYLINDERS FOR LABORATORY-CURED TEST SPECIMENS EXCEPT WHEN FIELD-CURED

TEST SPECIMENS ARE REQUIRED.

6. COMPRESSIVE—STRENGTH TESTS: ASTM C 39: ONE SET FOR EACH DAY'S POUR EXCEEDING 5 CU. YD. PLUS ADDITIONAL SETS FOR EACH 50 CU. YD. MORE THAN THE FIRST 25 CU. YD. OF EACH CONCRETE CLASS PLACED IN ANY ONE DAY; ONE SPECIMEN TESTED 7 DAYS. TWO SPECIMENS TESTED AT 28 DAYS. AND ONE SPECIMEN RETAINED IN RESERVE FOR LATER TESTING IF

- (C) WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE STRENGTH TESTS FOR A GIVEN CLASS OF CONCRETE, CONDUCT TESTING FROM AT LEAST FIVE RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN FIVE ARE USED.
- (D) TEST RESULTS WILL BE REPORTED IN WRITING TO ARCHITECT, STRUCTURAL ENGINEER, READY-MIX PRODUCER, AND CONTRACTOR WITHIN 24 HOURS AFTER TESTS. REPORTS OF COMPRESSIVE STRENGTH TESTS SHALL CONTAIN THE PROJECT IDENTIFICATION NAME AND NUMBER. DATE OF CONCRETE PLACEMENT. NAME OF CONCRETE TESTING SERVICE. CONCRETE TYPE AND CLASS. LOCATION OF CONCRETE BATCH IN STRUCTURE, DESIGN COMPRESSIVE STRENGTH AT 28 DAYS, CONCRETE MIX PROPORTIONS AND
- (E) NONDESTRUCTIVE TESTING: IMPACT HAMMER, SONOSCOPE, OR OTHER NONDESTRUCTIVE DEVICE MAY BE PERMITTED BY SHALL NOT BE USED AS THE SOLE BASIS FOR ACCEPTANCE OR REJECTION.

MATERIALS, COMPRESSIVE BREAKING STRENGTH, AND TYPE OF BREAK FOR BOTH 7-DAY TESTS AND 28-DAY TEST.

(F) ADDITIONAL TEST:

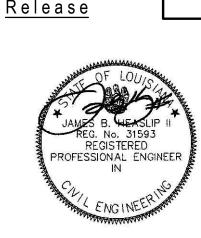
THE TESTING AGENCY WILL MAKE ADDITIONAL TESTS OF IN-PLACE CONCRETE WHEN TEST RESULTS INDICATE SPECIFIED CONCRETE STRENGTHS AND OTHER CHARACTERISTICS HAVE NOT BEEN ATTAINED IN THE STRUCTURE, AS DIRECTED BY ARCHITECT. TESTING AGENCY MAY CONDUCT TESTS TO DETERMINE ADEQUACY OF CONCRETE BY CORED CYLINDERS COMPLYING WITH ASTM C 42, OR BY OTHER METHODS



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Project No. 2216 Drawn By: JRN

**GENERAL** STRUCTURAL **NOTES** 

MY KNOWLEDGE AND BELIEF, THEY COMPLY WITH ALL LOCAL, REGIONAL AND NATIONAL REQUIREMENTS. I AM NOT OBSERVING THE WORK. ENGINEER: JAMES B HEASLIP AE PROJECT #: 22157 LICENSE NUMBER:

3500 N. CAUSEWAY BLVD., #200, METAIRIE, LA 70002 504.380.0800 OFFICE - INFO@AXISENGR.COM EMAIL THESE PLANS HAVE BEEN PREPARED BY ME OR UNDER MY CLOSE PERSONAL SUPERVISION AND TO THE BEST OF

## WOOD FRAMING NOTES:

- 1. APPLICABLE CODES OR STANDARDS:
  - ALL DESIGN, FABRICATION, TESTING, AND ERECTION SHALL BE IN ACCORDANCE WITH THE FOLLOWING CODES AND STANDARDS:
  - (A) IRC INTERNATIONAL RESIDENTIAL CODE (IRC)
  - (B) AWC NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS)
  - (C) AWC WOOD FRAME CONSTRUCTION MANUAL FOR ONE AND TWO-FAMILY DWELLINGS (WFCM)
  - (D) APA PLYWOOD DESIGN SPECIFICATION (PDS)
- 2. WALL SYSTEMS:

WALL SYSTEMS SHALL MEET THE SPECIFICATIONS LISTED IN THE PLAN NOTES (UNLESS NOTED OTHERWISE).

#### 3. MATERIALS:

MATERIALS SHALL MEET THE SPECIFICATIONS LISTED IN THIS SECTION (UNLESS NOTED OTHERWISE):

- (A) FRAMING LUMBER SHALL BE SOUTHERN PINE GRADE MARKED AND KILN DRIED, NO. 2.
- (B) ALL LUMBER IN CONTACT WITH CONCRETE OR MASONRY SHALL BE TREATED. LUMBER, PLYWOOD, PSL, OR OTHER STRUCTURAL WOOD ELEMENTS SHALL BE PRESSURE TREATED IN ACCORDANCE WITH AWPA.
- (C) FLOOR PLYWOOD SHEATHING SHALL BE 3/4" THICK.
- (D) WALL PLYWOOD SHEATHING SHALL BE 1/2" THICK.
- (E) ATTIC PLYWOOD SHEATHING SHALL BE 1/2" THICK.
- (F) ROOF PLYWOOD SHEATHING SHALL BE 5/8" THICK.
- (G) MEMBERS DESIGNATED AS "LVL" SHALL BE LAMINATED VENEER LUMBER HAVING PROPERTIES AND STRENGTHS EQUAL TO THE i-LEVELTRUSS JOIST COMPANY'S "MICROLLAM" OR APPROVED EQUAL.
- (H) MEMBERS DESIGNATED AS "PSL" SHALL BE PARALLEL STRAND LUMBER HAVING PROPERTIES AND STRENGTHS EQUAL TO THE I-LEVEL TRUSS JOIST COMPANY'S "PARALLAM" OR APPROVED EQUAL.
- (I) MEMBERS DESIGNATED AS "TJI" SHALL BE "I-SHAPED" WOOD JOIST LUMBER HAVING PROPERTIES AND STRENGTHS
- (J) MEMBERS DESIGNATED AS "PPG" SHALL BE GLULAM LUMBER HAVING PROPERTIES AND STRENGTHS EQUAL TO THE ANTHONY FOREST PRODUCTS COMPANY'S "POWER PRESERVED GLULAM" OR APPROVED EQUAL.
- (K) JOIST HANGERS, BEAM HANGERS, HURRICANE CLIPS, ANCHORS, AND CONNECTORS SHALL BE SUPPLIED BY SIMPSON STRONG—TIE CO., INC. OR APPROVED EQUAL AND ATTACHED WITH MANUFACTURER RECOMMENDATIONS.
- (L) HANGERS, CLIPS, CONNECTORS, ANCHORS, TIES, ETC. SHALL BE GALVANIZED.
- (M) HANGERS, CLIPS, CONNECTORS, ANCHORS, TIES, ETC. EXPOSED TO WEATHER, IN CONTACT WITH EARTH OR WATER, OR BELOW THE FIRST FLOOR LEVEL SHALL RECEIVE THE SIMPSON "Z-MAX" TRIPLE ZINC COATING OR APPROVED EQUAL.
- (N) STUD WALL BOTTOM PLATES CONNECTED TO CONCRETE SHALL BE SUPPLIED BY RAMSET OR APPROVED EQUAL.

#### 4. CONNECTIONS:

CONNECTIONS SHALL MEET THE SPECIFICATIONS LISTED IN THIS SECTION (UNLESS NOTED OTHERWISE):

EQUAL TO THE I-LEVEL TRUSS JOIST COMPANY'S "TRUS JOIST" OR APPROVED EQUAL.

- (A) WOOD MEMBERS (INCLUDING PLYWOOD SHEATHING OR BRACING) SHALL BE CONNECTED OR FASTENED WITH STEEL NAILS, SCREWS, OR BOLTS. ALL EXPOSED NAILS, SCREWS, OR BOLTS SHALL BE POLYMER COATED OR GALVANIZED.
- (B) NO STAPLES SHALL BE PERMITTED.
- (C) WOOD CONNECTIONS SHALL BE IN ACCORDANCE WITH THE FASTENING SCHEDULE LISTED IN IRC 2015 TABLE R602.3.
- (D) MEMBER END PIECES, JOINTS, OR SPLICES SHALL BE OVER SUPPORTS.
- (E) MULTIPLE PIECES OF LUMBER OR MANUFACTURED WOOD PRODUCTS USED TO FORM BEAM OR HEADER MEMBERS SHALL BE ATTACHED TOGETHER WITH (2) ROWS OF 12d NAILS SPACED AT 12" FOR PIECES UP TO 12" DEEP. ALL OTHER PIECES SHALL BE ATTACHED TOGETHER WITH (3) ROWS OF 12d NAILS SPACED AT 12".
- (F) MULTIPLE PIECES OF LUMBER USED TO FORM PACKED STUDS SHALL BE ATTACHED TOGETHER WITH (2) ROWS OF NAILS SPACED AT 8".
- (G) PLYWOOD WALL SHEATHING SHALL HAVE SOLID BLOCKING AT ALL HORIZONTAL JOINTS.
- (H) PLYWOOD ROOF SHEATHING VERTICAL JOINTS SHALL BE STAGGERED EVERY 4 FEET OR LESS.
- (I) FLOOR JOISTS SHALL HAVE BRIDGING AT 8'-0" O.C. (MAX.).
- (J) BOTTOM PLATE OF STUD WALLS TO CONCRETE SHALL BE CONNECTED WITH 1/4" RAMSETS AT 16" O.C.
- (K) PRE-ENGINEERED STRUCTURAL MEMBERS INCLUDING PSL, PPG, LVL, ETC. SHALL BE ERECTED AND BRACED IN ACCORDANCE WITH MANUFACTURER SPECIFICATIONS.

#### 5. OPENINGS:

OPENINGS SHALL MEET THE SPECIFICATIONS LISTED IN THIS SECTION (UNLESS NOTED OTHERWISE):

- (A) OPENINGS IN WALLS SHALL HAVE HEADERS CONSISTING OF A MINIMUM OF TWO 2x12's OR THREE 2x10's (4'-0" MAX.).
- (B) OPENINGS IN EXTERIOR WALLS SHALL BE IN ACCORDANCE WITH THE FULL HEIGHT STUD REQUIREMENTS LISTED IN WFCM TABLE 3.23C.
- (C) FULL HEIGHT STUDS MAY BE REDUCED IN ACCORDANCE WITH THE REQUIREMENTS LISTED IN WFCM TABLE 3.23D.
- (D) JACK STUDS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS LISTED IN WFCM TABLE 3.22F.

#### 6. PRE-ENGINEERED WOOD TRUSSES:

PRE-ENGINEERED WOOD TRUSSES SHALL MEET THE SPECIFICATIONS LISTED IN THIS SECTION (UNLESS NOTED OTHERWISE):

- (A) TRUSSES SHALL BE DESIGNED TO MEET THE REQUIREMENTS OF THE NATIONAL DESIGN SPECIFICATION BY THE NFPA AND THE IBC.
- (B) THE DESIGN, CONFIGURATION, LAYOUT, SPACING, ETC. OF ALL TRUSSES SHALL BE COORDINATED BY THE CONTRACTOR AND TRUSS MANUFACTURER WITH THE MECHANICAL EQUIPMENT, DUCTWORK, AND ARCHITECTURAL DRAWINGS.
- (C) THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS OF THE TRUSSES FOR REVIEW BY THE ENGINEER OF RECORD. THE SHOP DRAWINGS SHALL BE STAMPED BY A PROFESSIONAL ENGINEER.
- (D) EVERY TRUSS MEMBER SHALL BE SIZED AND BRACED SO THAT THE RATIO OF ITS LENGTH TO ITS DEPTH AND ITS THICKNESS (L/D) IS LESS THAN 50. MANY OF THE CEILINGS WILL NOT BE RIGIDLY ATTACHED TO THE TRUSS BOTTOM CHORDS. WHERE THIS OCCURS, LATERAL BRACING MUST BE DESIGNED AND INSTALLED ON THE BOTTOM CHORDS TO MEET THE LESS THAN 50 (L/D) RATIO.
- (E) ATTACH TRUSSES TO SUPPORTS WITH SPECIFIED GALVANIZED METAL CONNECTORS.
- (F) IF SHEET METAL CONNECTORS ARE USED, THE CONNECTORS AT EVERY JOINT SHALL BE SIZED FOR TWICE THE CALCULATED LOAD AT THE JOINT.

## POST TENSION NOTES:

- 1. TENDONS AND BARS SHALL BE SECURELY SUPPORTED TO PREVENT BOTH VERTICAL AND HORIZONTAL MOVEMENT DURING
- ALLOW 8" DIA. CENTERED ON TENDON AXIS BY 36" LENGTH FOR STRESSING EQUIPMENT CLEARANCE.
- 3. POST-TENSIONING WORK & MATERIALS SHALL BE PER PROJECT SPECIFICATION SECTION 03500" UNBONDED POST-TENSIONED FOUNDATION SYSTEMS ON GRADE."
- 4. PRE-STRESSED STEEL SHALL CONSIST OF SEVEN WIRE STRESS RELIEVED STRAND CONFORMING TO ASTM A-416 LOW-RELAXATION STRAND. MINIMUM ULTIMATE TENSILE STRENGTH SHALL BE 270,00 P.S.I. STRANDS SHALL BE COATED WITH A PERMANENT RUST PREVENTATIVE LUBRICANT AND A PLASTIC SHEATH. TENDONS SHALL BE 1/2" U.N.O. EACH TENDON SHALL BE INITIALLY STRESSED TO 33.0 KIPS AND SHALL BE ANCHORED AT 29.0 KIPS.
- 5. POST—TENSION SYSTEM SHALL BE FURNISHED, PLACED, AND STRESSED BY A FIRM SPECIALIZING IN POST—TENSION SYSTEMS. POST TENSION SUPPLIERS SHALL BE PTI CERTIFIED. POST—TENSION CONTRACTOR SUPERVISOR AND 50% OF THE INSTALLATION PERSONNEL MUST BE CERTIFIED AS HAVING COMPLETED THE PTI LEVEL 1 FIELD FUNDAMENTALS PROGRAM. ALSO, ALL PERSONNEL INVOLVED IN THE STRESSING OPERATION MUST BE CERTIFIED AS HAVING COMPLETED THE PTI LEVEL 1 FIELD FUNDAMENTALS PROGRAM.
- 6. TENDONS SHOULD BE STRESSED NO LATER THAN 10 DAYS AND NO EARLIER THAN 6 DAYS AFTER PLACEMENT OF CONCRETE UNLESS CONCRETE COMPRESSIVE STRENGTH CAN BE VERIFIED SOONER.
- 7. TENDONS AND BARS SHALL BE SECURELY SUPPORTED TO PREVENT BOTH VERTICAL AND HORIZONTAL MOVEMENT DURING PLACEMENT OF CONCRETE. TENDON SUPPORTS SHALL BE AT 4'-0" MAX. TENDON SUPPORTS SHALL NOT PENETRATE THE WARDER BARRIER
- 8. TENDON LOCATION AT THE END OF GRADE BEAM SHALL BE 5" MINIMUM TO 6" MAXIMUM FROM THE TOP OF SLAB TO CENTRAL GRAVITY OF TENDONS (SEE SECTIONS THIS DRAWING).
- 9. EXPOSED STRESSING RECESSES SHALL BE FILLED FLUSH WITH A NON-SHRINK GROUT. THIS WORK SHALL BE PERFORMED AS SOON AS PRACTICAL AFTER STRESSING BY THE CONTRACTOR, BUT NO LATER THAN 7 DAYS AFTER STRESSING.
- 10. TENDONS IN EXCESS OF 110' IN LENGTH ARE INTENDED TO BE STRESSED TO 33.0 KLPS FROM BOTH SIDES.
- 11. POST-TENSION TENDONS MAY BE STRESSED IN ANY SEQUENCE AND THE DEAD AND LIVE ENDS OF THE TENDONS MAY BE REVERSED FROM THAT SHOWN AT THE CONVENIENCE OF THE POST-TENSION CONTRACTOR.
- 12. PLANS FOR PIPES, CONDUITS, THIMBLES, ETC., TO PASS THROUGH CONCRETE SLAB OR BEAM, MUST NOT CONFLICT WITH REINFORCING. INSTALLATION SEQUENCE OF CONVENTIONAL AND POST—TENSION REINFORCING MUST BE COORDINATED BY GENERAL CONTRACTOR & INSTALLER. WHEN ANY CONFLICT OCCURS BETWEEN LOCATION OF TENDON AND LOCATION OF MILD STEEL REINFORCING, LOCATION OF TENDON SHALL TAKE PRECEDENCE.
- 13. TENDONS SHALL NOT BE PLACED WITHIN 6" OF A SLAB PENETRATION, OPENING, OR EXTERIOR EDGE.
- 14. CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATION OF ALL POST—TENSIONING TENDONS WHEN USING EXPANSION BOLTS, ADHESIVE ANCHORS, OR POWDER ACTUATED FASTENERS. THE CONTRACTOR SHALL LOCATE TENDONS WITH SPECIALIZED EQUIPMENT SUCH AS BAR METERS AND/OR PACHOMETERS FOR THIS PURPOSE
- 15. THE STRESSING OPERATION MUST BE UNDER THE IMMEDIATE CONTROL OF A PERSON EXPERIENCED IN THIS TYPE OF WORK. THIS PERSON MUST MAINTAIN A CLOSE CHECK AND A RIGID CONTROL OF ALL OPERATIONS. IF ANY PROBLEMS OCCUR DURING THE STRESSING OPERATION (I.E. BROKEN WIRES, ERRATIC ELONGATIONS, ETC.) DISCONTINUE THE STRESSING OPERATION AND CONTACT THE ENGINEER IMMEDIATELY.
- 16. THE STRESSING MUST NOT COMMENCE UNTIL CONCRETE TEST CYLINDERS, CURED UNDER JOBSITE CONDITIONS, HAVE BEEN TESTED AND INDICATE THAT THE CONCRETE HAS REACHED A MINIMUM STRENGTH OF 2200 PSI.
- 17. THE POST—TENSIONING OPERATION SHALL BE CONDUCTED SO THAT ACCURATE ELONGATION OF THE PRESTRESSING STEEL CAN BE RECORDED AND COMPARED WITH THE CALCULATED ELONGATIONS. POST TENSION CONTRACTOR SHALL DEVELOP TENDON SHOP DRAWING WITH TENDON NUMBERS, LENGTHS, AND REQUIRED TENDON ELONGATION. RECORDS SHALL BE KEPT OF ALL JACKING FORCES AND ELONGATIONS. MEASURED ELONGATIONS NOT WITHIN 10% OF THE CALCULATED ELONGATION SHALL BE RESOLVED BY THE STRUCTURAL ENGINEER WITH THE ASSISTANCE OF MANUFACTURER.
- 18. ALL INSERTS FOR CARPENTRY WORK MUST BE CAST IN PLACE. IF ADDITIONAL FASTENERS ARE REQUIRED, POWER-DRIVEN FASTENERS WILL BE PERMITTED ONLY WHERE CONCRETE SPALLING AND TENDON DAMAGE CAN BE PROHIBITED. CONTRACTORS MUST LOCATE TENDONS AT THE SURFACE ON THE SLAB BEFORE DRIVING FASTENERS.
- 19. IF THE TENDON SHEATHING IS DAMAGED OR REMOVED FOR APPROXIMATELY 6" OR MORE, IT SHOULD BE RESHEATHED TO PREVENT BONDING OF THE CONCRETE TO THE STRAND.
- 20. CONCRETE SHALL BE WELL CONSOLIDATED, ESPECIALLY IN THE VICINITY OF TENDON ANCHORAGES.
- 21. COORDINATE STRUCTURAL DRAWINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ALL OPENINGS, INSERTS, AND ANY OTHER RELATED ITEMS.
- 22. FORMS TO BE STRIPPED NO LATER THAN (6) DAYS AFTER PLACEMENT OF CONCRETE.
- 23. THE OWNER, BUILDER, GENERAL CONTRACTOR, AND/OR HOMEOWNER SHALL VERIFY ALL DROPS, OFFSETS, BRICK LEDGES, BLOCK OUTS, DEPRESSIONS, ETC. TO ARCHITECTURAL PLANS. IF ANY DISCREPANCIES OCCUR, CONTACT THE ENGINEER OF RECORD IMMEDIATELY PRIOR TO COMMENCING WORK AND ORDERING OF THE POST TENSION CABLES. THE OWNER, BUILDER, GENERAL CONTRACTOR, AND/OR HOMEOWNER SHALL BE RESPONSIBLE FOR COORDINATING OF THE STRUCTURAL DRAWINGS WITH ALL OTHER DRAWINGS.
- 24. LOADING OF THE SLAB PRIOR TO THE TENSIONING SHALL NOT BE DONE WITHOUT THE APPROVAL AND DIRECTION OF THE DESIGN ENGINEER. ONLY WOOD STRUCTURES CAN COMMENCE BEING FRAMED PRIOR TO STRESSING, UP TO THE BLACK IN
- 25. IT IS RECOMMENDED THAT 6x6 2.9/2.9 WWF BE INSTALLED AT THOSE CONCRETE AREAS THAT ARE TO BE LEFT EXPOSED, SUCH AS SCORED STAINED SEALED CONCRETE AND/OR AREAS TO RECEIVE BRITTLE FLOORING MATERIAL.
- 26. FLOORING CONTRACTOR MUST USE AN ELASTIC BOND BREAKER ON ANY CONCRETE SURFACE AREA THAT MAY SHOW CONCRETE CRACKING TO PREVENT BONDING OF THE BRITTLE FLOORING MATERIALS TO THE FOUNDATION. ENGINEER AND POST TENSION INSTALLER NOT RESPONSIBLE FOR REFLECTIVE CRACKING, EXPANSION AND CONTRACTION IF NOT PROPERLY INSTALLED.

## DESIGN INFORMATION:

- 1. DESIGN LOADS SHALL MEET THE SPECIFICATIONS LISTED IN THIS SECTION (UNLESS NOTED OTHERWISE).
  - (A) DESIGN BUILDING CODE 2015 INTERNATIONAL RESIDENTIAL CODE (IRC)
  - (B) <u>DESIGN GRAVITY LOADS:</u>

IV CIVIVIII LONDO.	
FIRST FLOOR	DL = 50 PS LL = 40 PS
SECOND FLOOR	DL = 10 PS LL = 30 PS
ATTIC	DL = 10 PS LL = 20 PS
ROOF	DL = 15 PS LL = 20 PS

(C) WIND LOADS SHALL BE IN ACCORDANCE WITH ASCE 7-10:

#### MAIN WIND FORCE RESISTING SYSTEM

PARAMETER	VALUE	REFERENCE
RISK CATEGORY BASIC WIND SPEED		TABLE 1.5-1 FIGURE 26.5-1B
DIRECTIONALITY EXPOSURE CATEGORY TOPOGRAPHIC FACTOR GUST EFFECT FACTOR ENCLOSURE CLASSIFICATION INTERNAL PRESSURE COEFFICIENT VELOCITY	Kd = 0.85 B Kzt = 1.0 0.85 ENCLOSED CGpi = +/-0.18 qh = 31.15 PSF	FIGURE 26.6-1 SECTION 26.7 FIGURE 26.8-1 SECTION 26.9 SECTION 26.10 SECTION 26.11 SECTION 28.3.2



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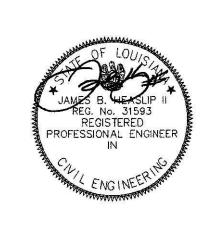
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GENERAL STRUCTURAL NOTES

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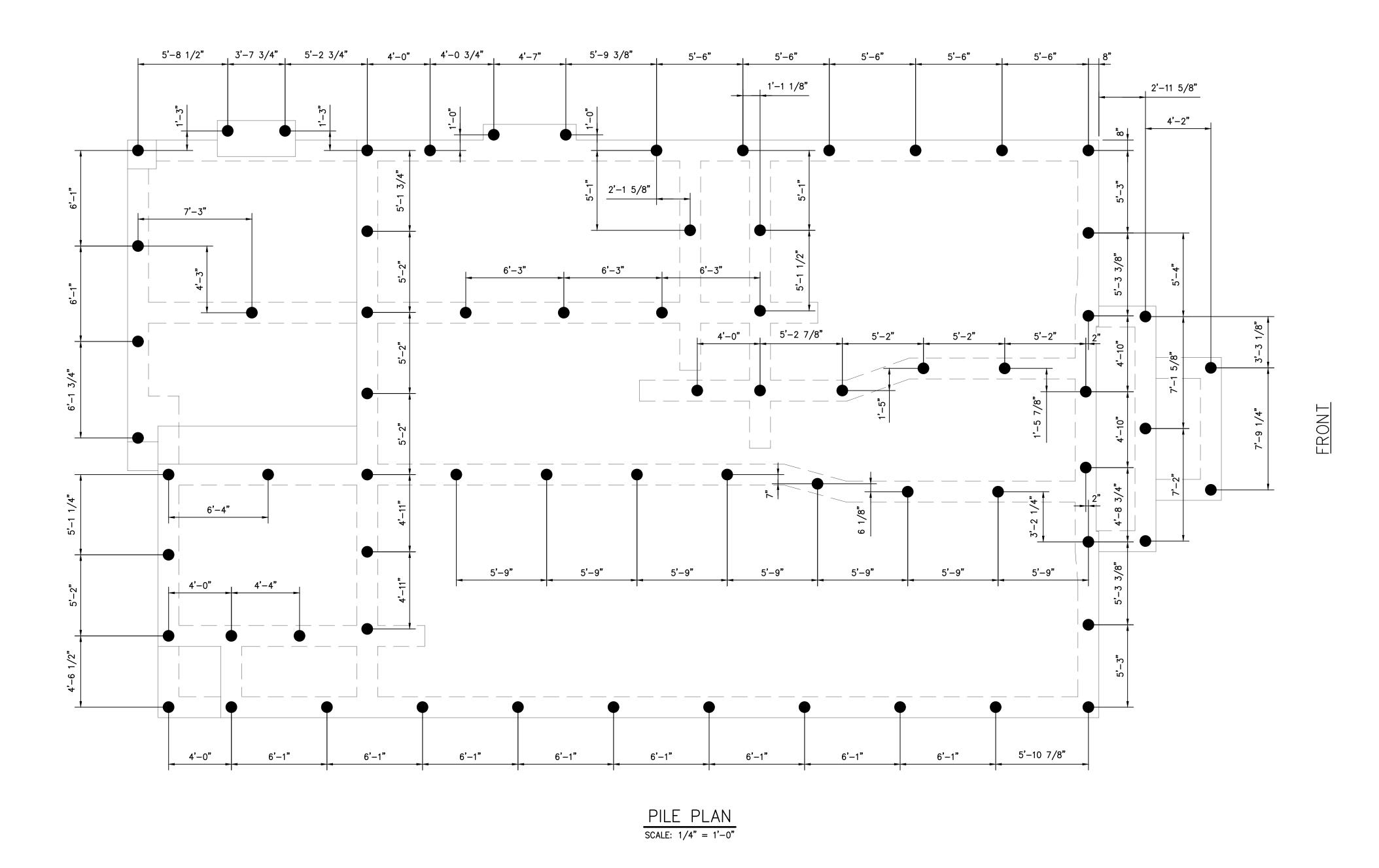
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ENGINEER: JAMES B HEASLIP AE PROJECT #: 22157

I AM NOT OBSERVING THE WORK.

LICENSE NUMBER:

S1.0B



## PLAN NOTES:

- 1. FOR ALL NOTES, SEE DRAWINGS S1.0A S1.0B.
- 2. FOR ALL PLAN NOTES, SEE DRAWING S1.3.
- 3. TOTAL PILES = 69



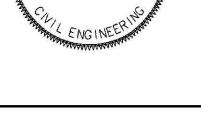
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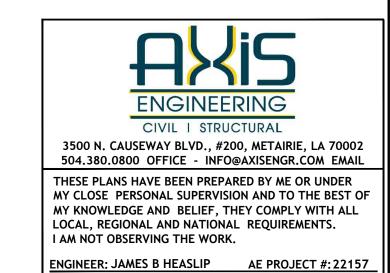
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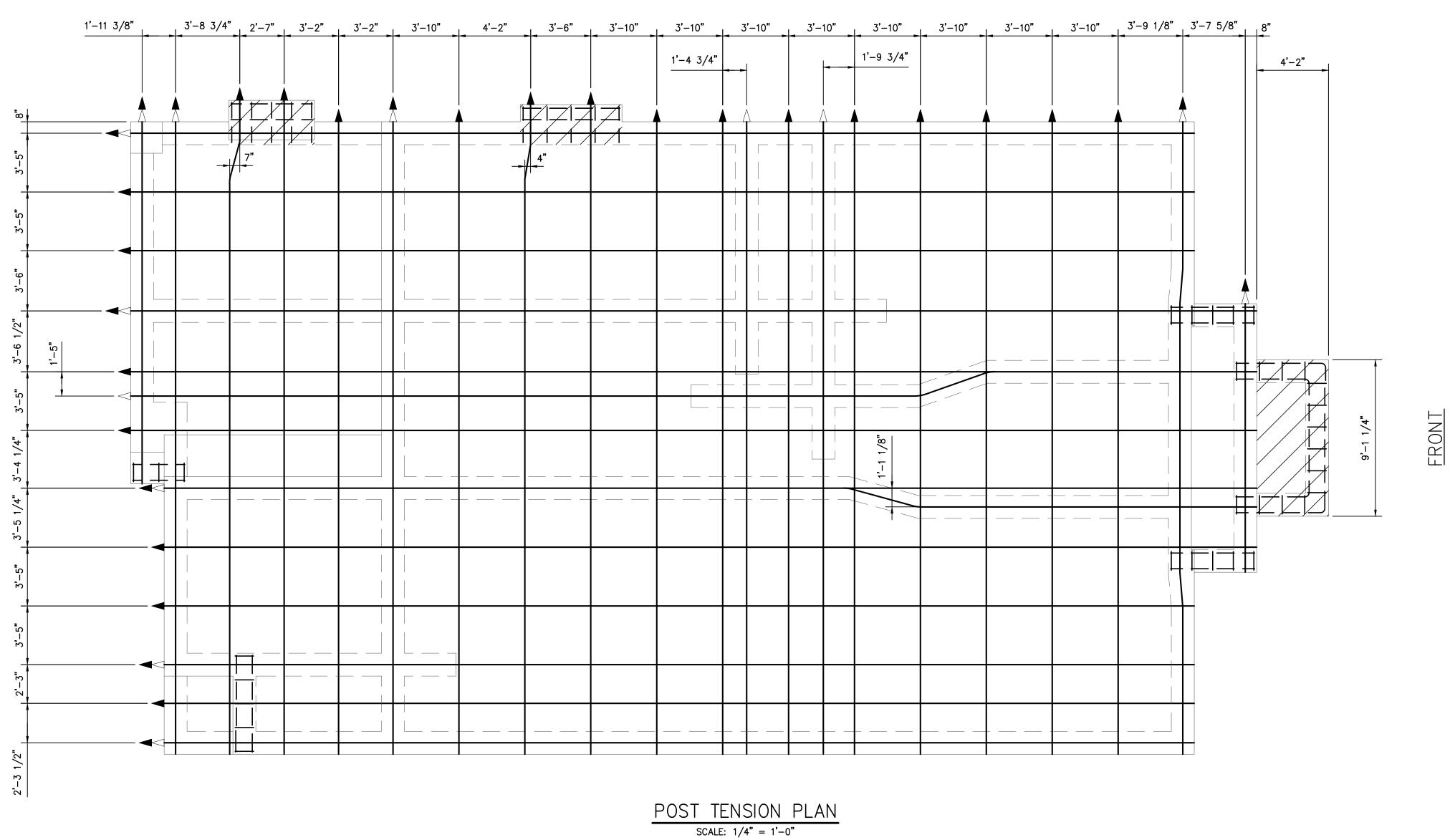
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PLAN





## PLAN NOTES:

- 1. FOR ALL NOTES, SEE DRAWINGS S1.0A S1.0B.
- 3. 
  → SYMBOL ON PLAN INDICATES BOTTOM BEAM LEVEL TENDON.
- SYMBOL ON PLAN INDICATES LOCATION OF CONVENTIONAL REBAR PLACEMENT (SEE SECTIONS FOR DETAILS).



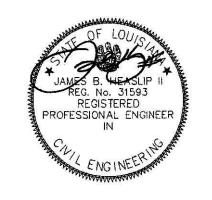
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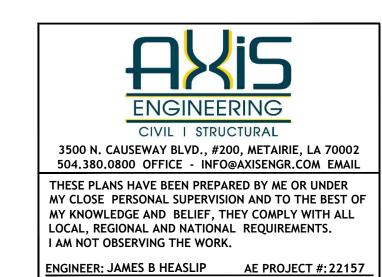
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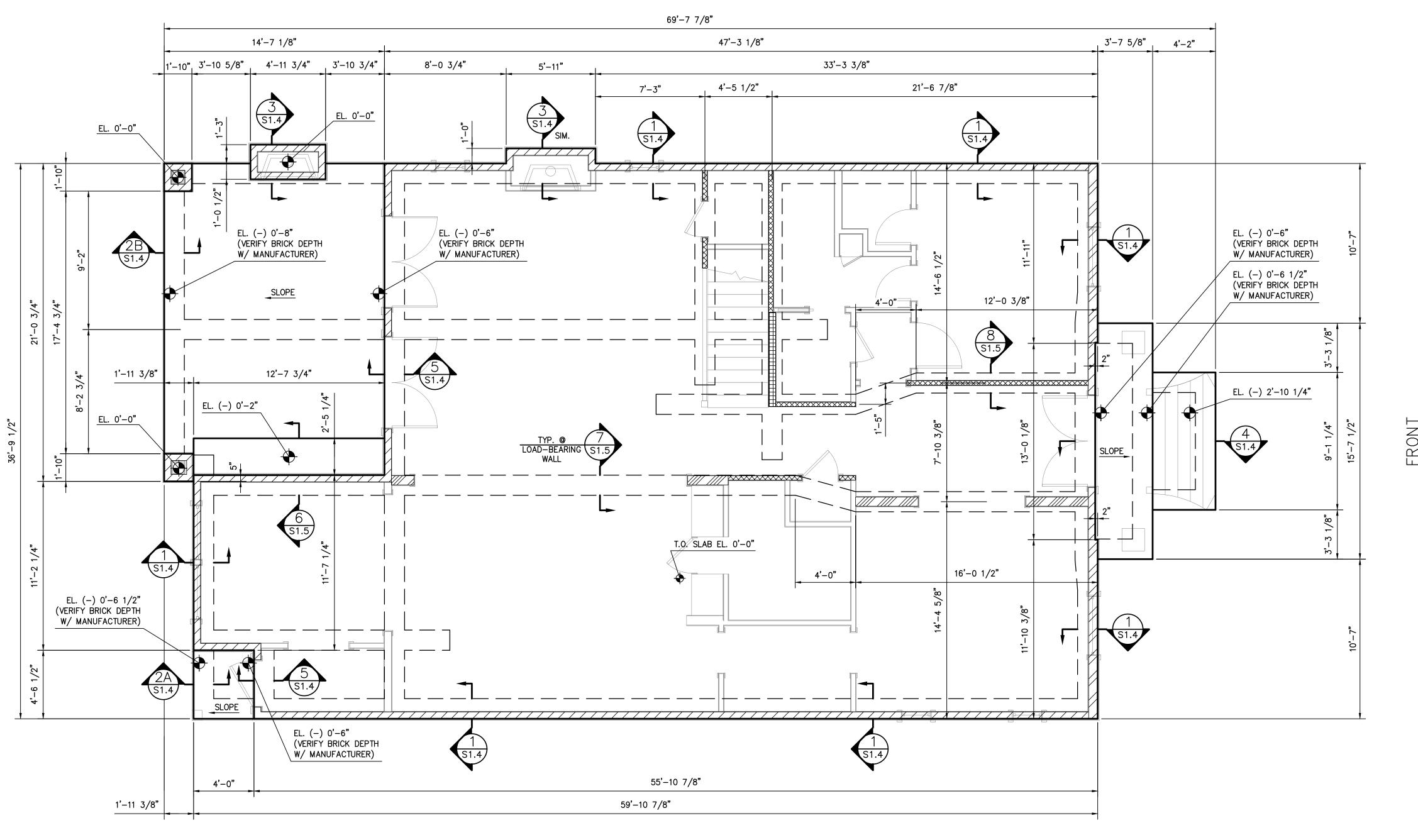
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POST **TENSION** PLAN





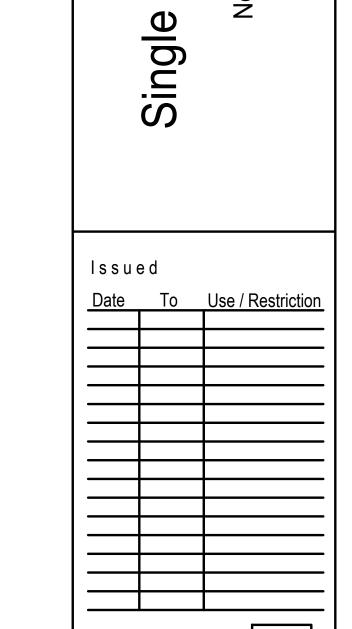
SCALE: 1/4" = 1'-0"

## PLAN NOTES:

- 1. FOR ALL NOTES, SEE DRAWING S1.0A S1.0B.
- 2. TOP OF 4" SLAB ELEVATION 0'-0"; (REF.) (-) 3.65' N.A.V.D. (SEE SURVEY).
- 3. PRIOR TO CONSTRUCTION, CONTRACTOR TO VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
- 4. MEASUREMENTS ARE TO EDGE OF FRAMING. SEE ARCHITECTURAL DRAWINGS FOR FINISHED DIMENSIONS.
- 5. ///// SYMBOL ON PLAN INDICATES 2x8 EXTERIOR LOAD-BEARING WALL SYSTEM:
  - A. 2x8 STUD @ 16" O.C. (MAX.) TREATED 2x8 BOTTOM PLATE
  - DOUBLE 2x8 TOP PLATE
  - D. SHEATHING SEE WOOD FRAMING NOTES E. BLOCKING @ 48" O.C. (MAX.)
- 6. SYMBOL ON PLAN INDICATES 2x6 EXTERIOR LOAD-BEARING WALL SYSTEM:
  - A. 2x6 STUD @ 16" O.C. (MAX.)
    B. TREATED 2x6 BOTTOM PLATE

  - C. DOUBLE 2x6 TOP PLATE D. SHEATHING - SEE WOOD FRAMING NOTES
  - E. BLOCKING @ 48" O.C. (MAX.)
- 7. //// /// SYMBOL ON PLAN INDICATES 2x8 INTERIOR LOAD-BEARING WALL SYSTEM:
  - A. 2x8 STUD @ 16" O.C. (MAX.)
  - B. TREATED 2x8 BOTTOM PLATE DOUBLE 2x8 TOP PLATE
  - D. BLOCKING @ 48" O.C. (MAX.)
- - A. 2x6 STUD @ 16" O.C. (MAX.)
    B. TREATED 2x6 BOTTOM PLATE
  - C. DOUBLE 2x6 TOP PLATE
  - D. BLOCKING @ 48" O.C. (MAX.)
- 9. SYMBOL ON PLAN INDICATES 2x4 INTERIOR LOAD—BEARING WALL SYSTEM:
  - A. 2x4 STUD @ 16" O.C. (MAX.) B. TREATED 2x4 BOTTOM PLATE
  - C. DOUBLE 2x4 TOP PLATE
  - D. BLOCKING @ 48" O.C. (MAX.)

FOUNDATION	PLAN



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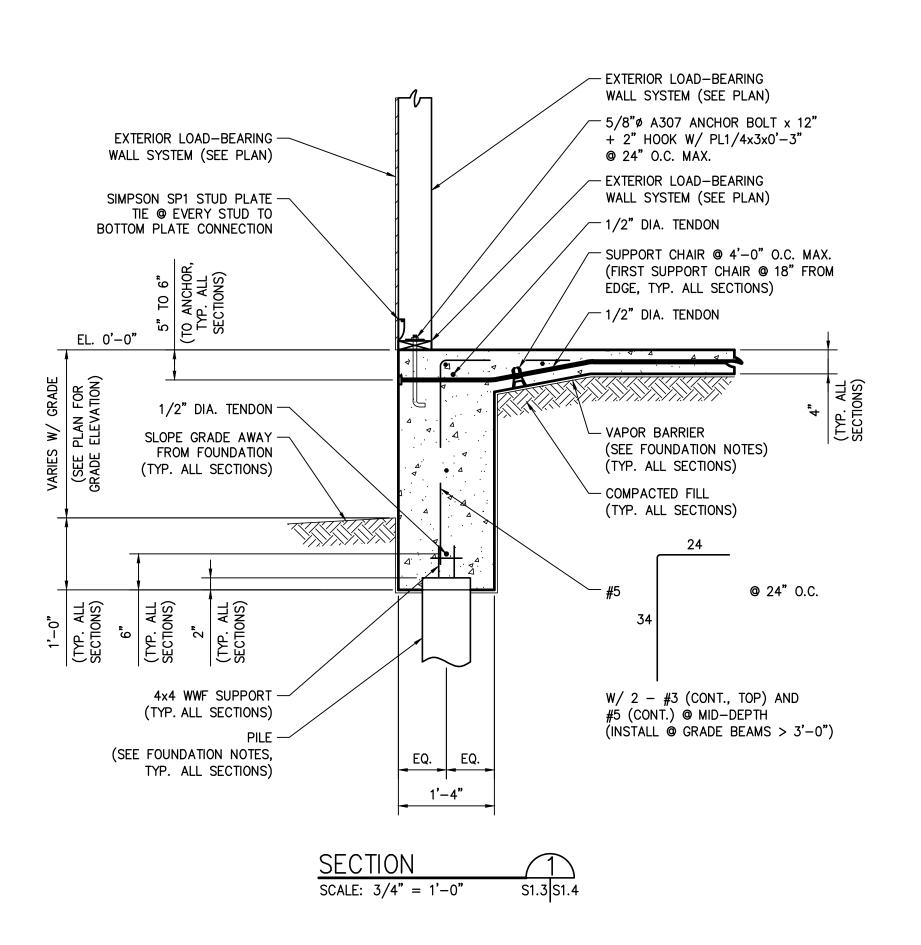
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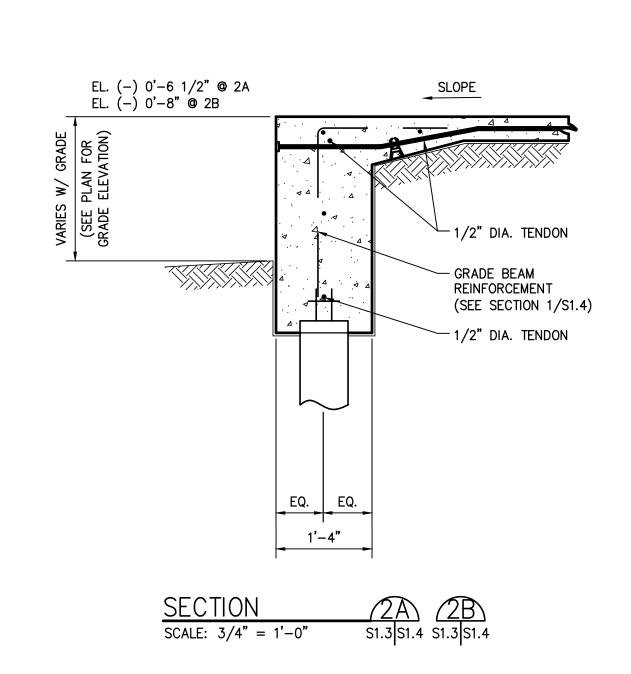
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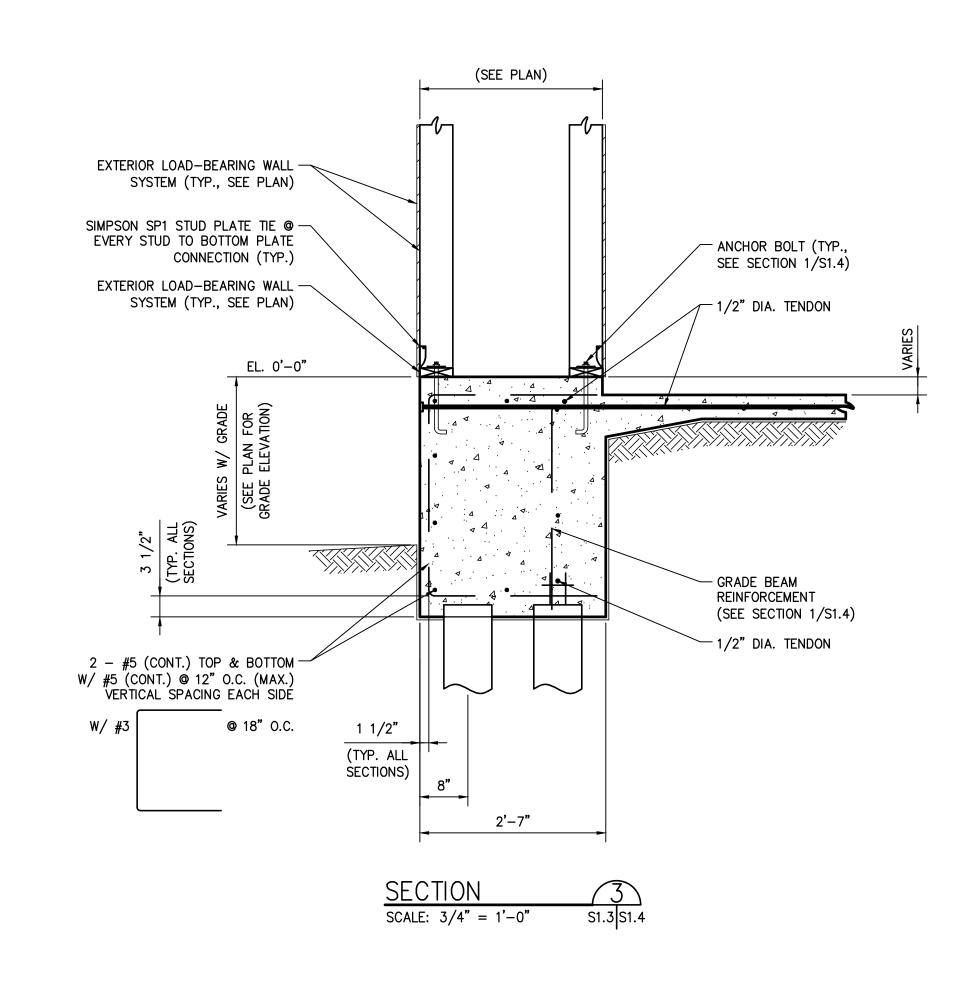
**FOUNDATION** PLAN

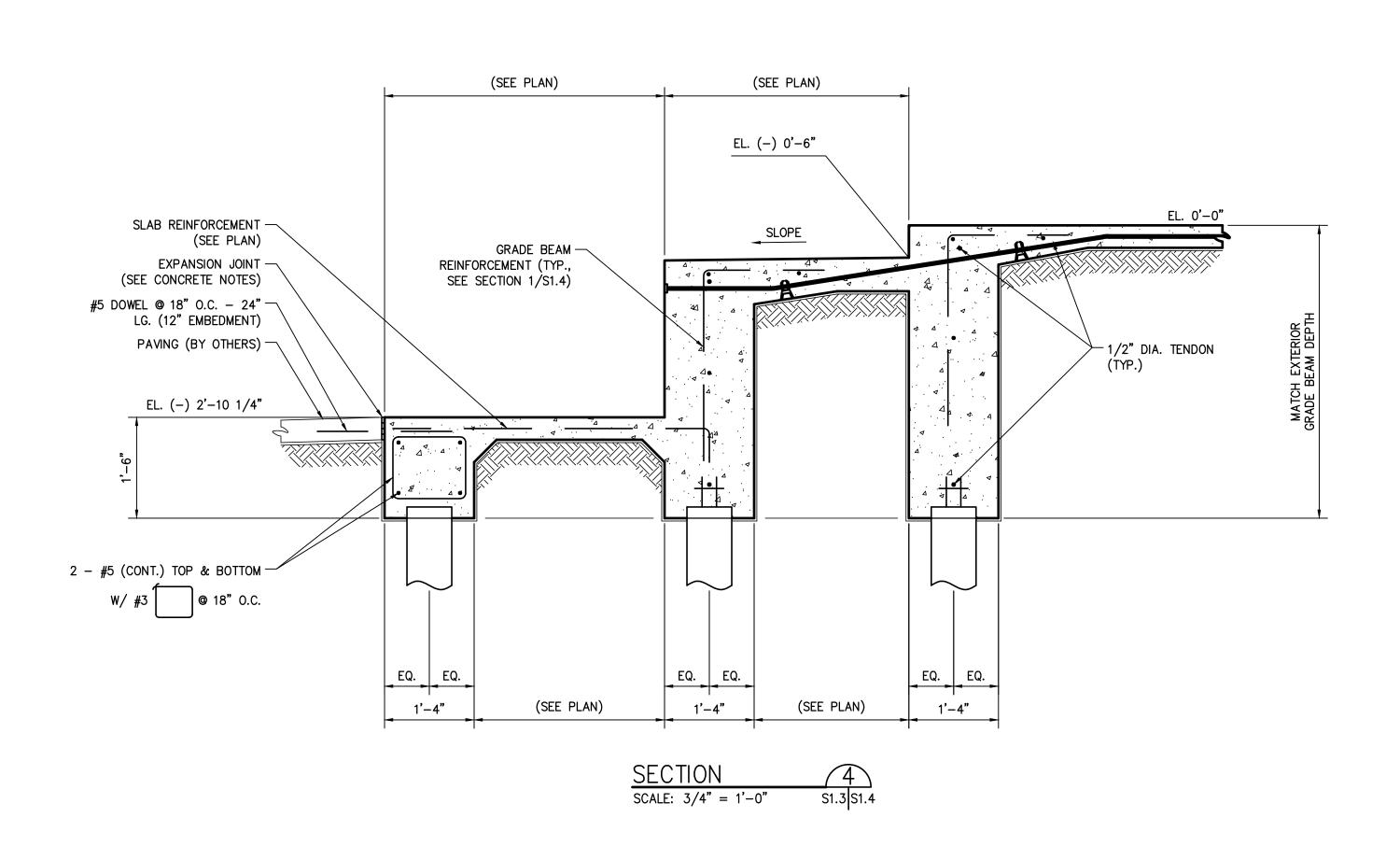
ENGINEER: JAMES B HEASLIP AE PROJECT #: 22157

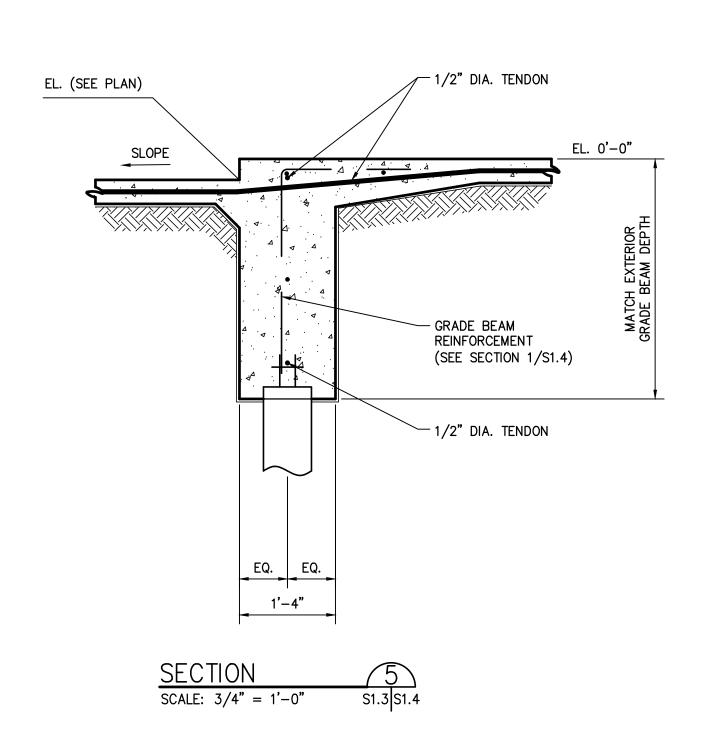
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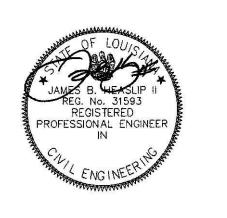




Residence

Orleans, Single

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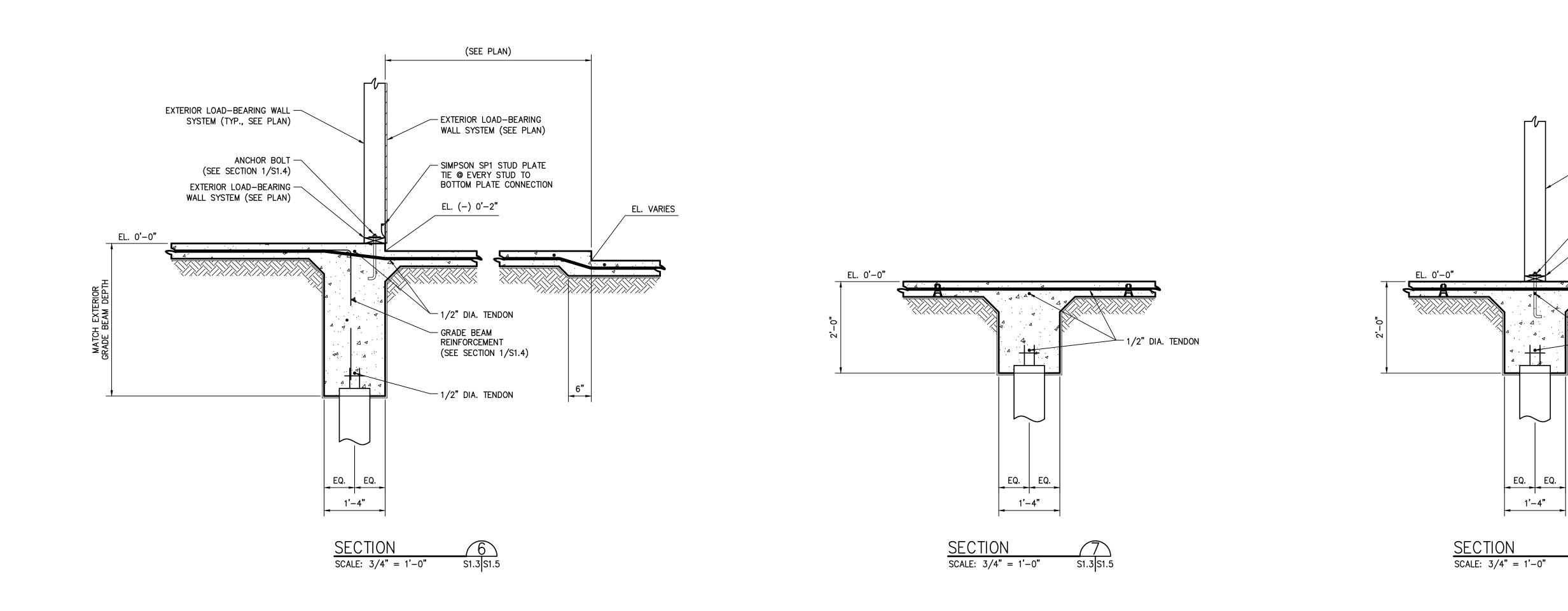


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**FOUNDATION** SECTIONS

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1/2" DIA. TENDON

└─ 1/2" DIA. TENDON

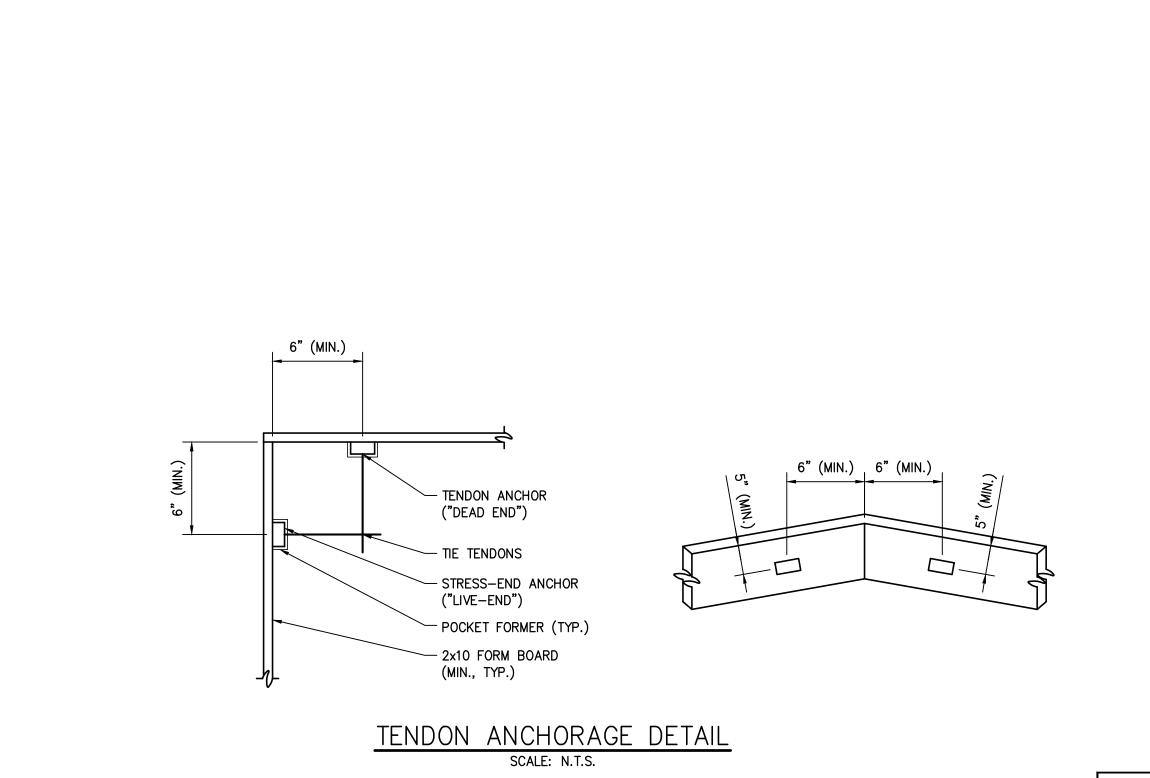
6:1 SLOPE (OR 36" MIN.)

TRANSITIONAL GRADE BEAM SECTION

SCALE: N.T.S.

3 − #5 x 10'−0" LG. — @ 6" O.C.

TYPICAL CORNER DETAIL





2134 Magazine St, Suite 200 New Orleans, Louisiana 70130

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INTERIOR LOAD—BEARING WALL SYSTEM (SEE PLAN)

ANCHOR BOLT
(SEE SECTION 1/S1.4)

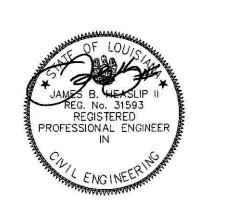
- 1/2" DIA. TENDON

✓ INTERIOR LOAD—BEARING WALL SYSTEM (SEE PLAN)

Family Residence 6544 Vicksburg St. 544 Vicksk Orleans, Single

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FOUNDATION **SECTIONS AND** DETAILS

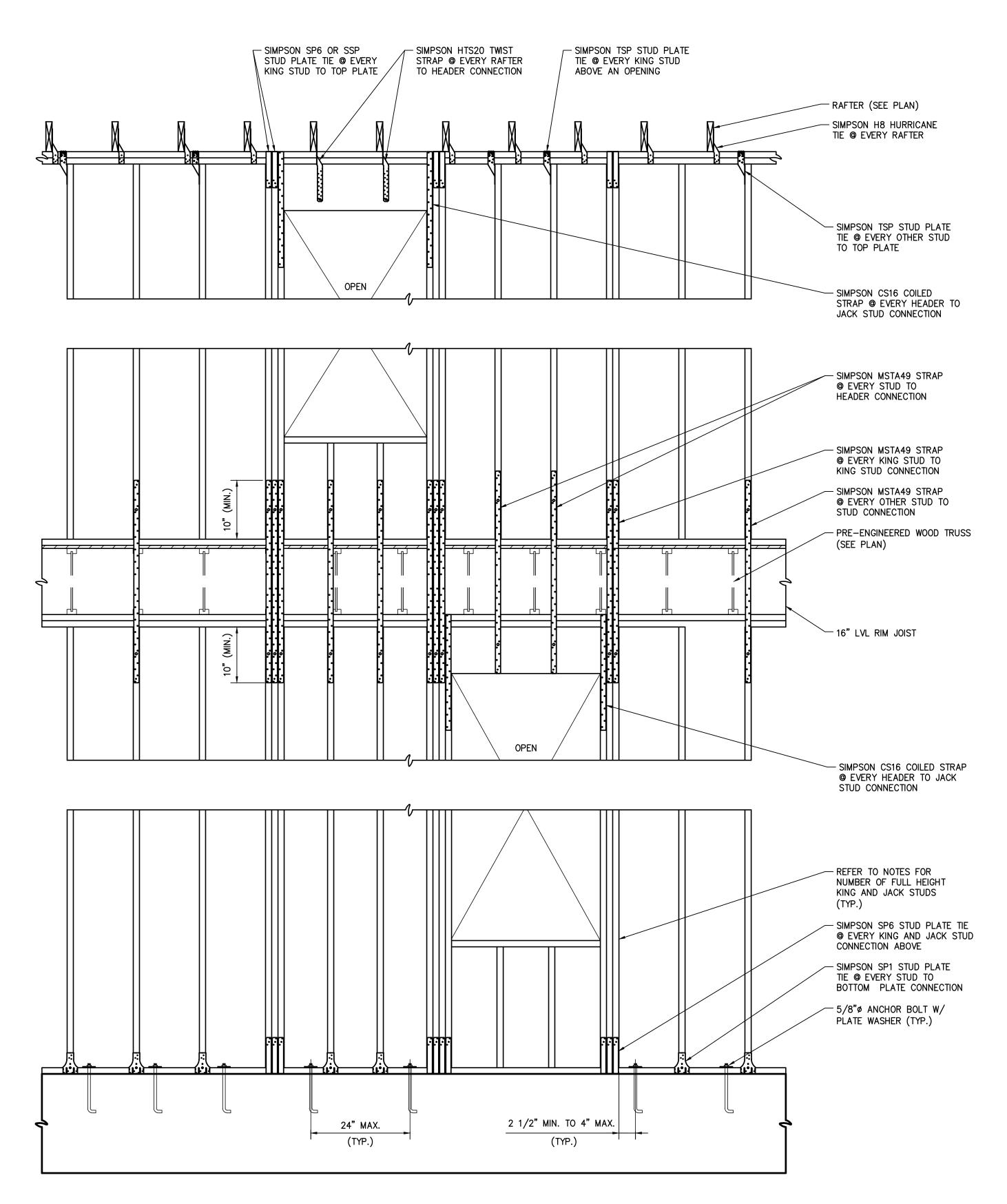
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S1.5



CONTINUOUS LOAD PATH @ TWO-STORY STACKED AND OFFSET OPENINGS DETAIL

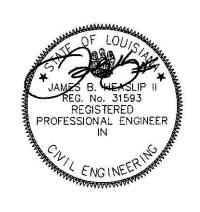


Family Residence 6544 Vicksburg St.

Single

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<u>Progress</u> Release



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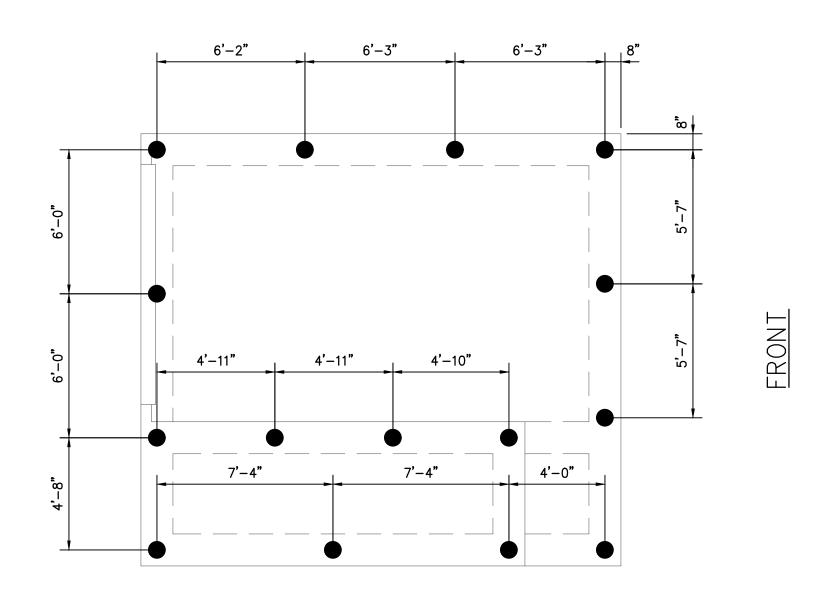
FRAMING DETAIL

S1.6

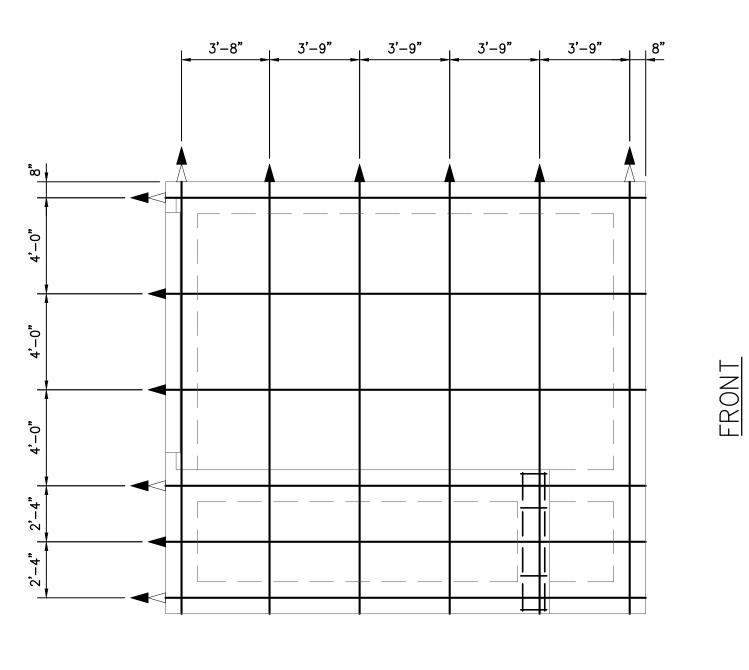
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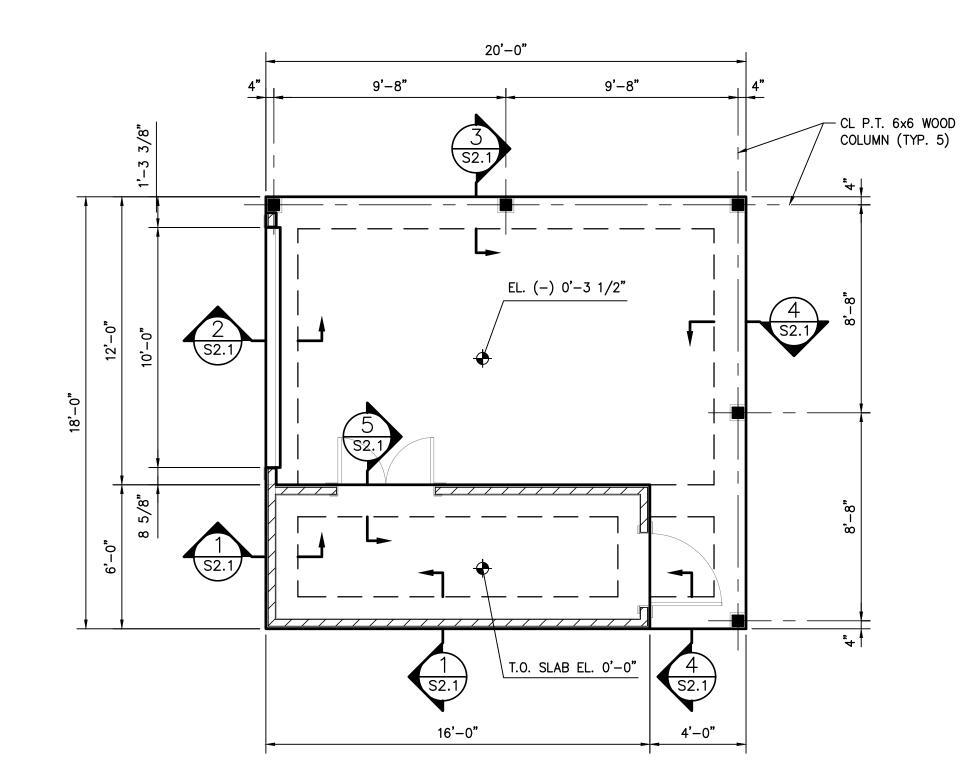
LICENSE NUMBER:



CARPORT POST TENSION PLAN SCALE: 1/4" = 1'-0"



CARPORT POST TENSION PLAN SCALE: 1/4" = 1'-0"



CARPORT FOUNDATION PLAN SCALE: 1/4" = 1'-0"

## PLAN NOTES:

- 1. FOR ALL NOTES, SEE DRAWING S1.0A S1.0B.
- 2. TOP OF 4" SLAB ELEVATION 0'-0"; (REF.) (-) 4.36' N.A.V.D. (SEE SURVEY).
- PRIOR TO CONSTRUCTION, CONTRACTOR TO VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
- 4. MEASUREMENTS ARE TO EDGE OF FRAMING. SEE ARCHITECTURAL DRAWINGS FOR FINISHED DIMENSIONS.
- 5. TOTAL PILES = 16
- 6. ■ SYMBOL ON PLAN INDICATES SLAB LEVEL TENDON.
- 7. ✓ SYMBOL ON PLAN INDICATES BOTTOM BEAM LEVEL TENDON.
- 8. T T SYMBOL ON PLAN INDICATES LOCATION OF CONVENTIONAL REBAR PLACEMENT (SEE SECTIONS FOR DETAILS).
- 9. SYMBOL ON PLAN INDICATES 2x4 EXTERIOR LOAD-BEARING WALL SYSTEM:
  - A. 2x4 STUD @ 16" O.C. (MAX.) B. TREATED 2x4 BOTTOM PLATE

  - C. DOUBLE 2x4 TOP PLATE
    D. SHEATHING SEE WOOD FRAMING NOTES
    E. BLOCKING @ 48" O.C. (MAX.)

Prog	ress		
Rele	ase		
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ARCHITECTS

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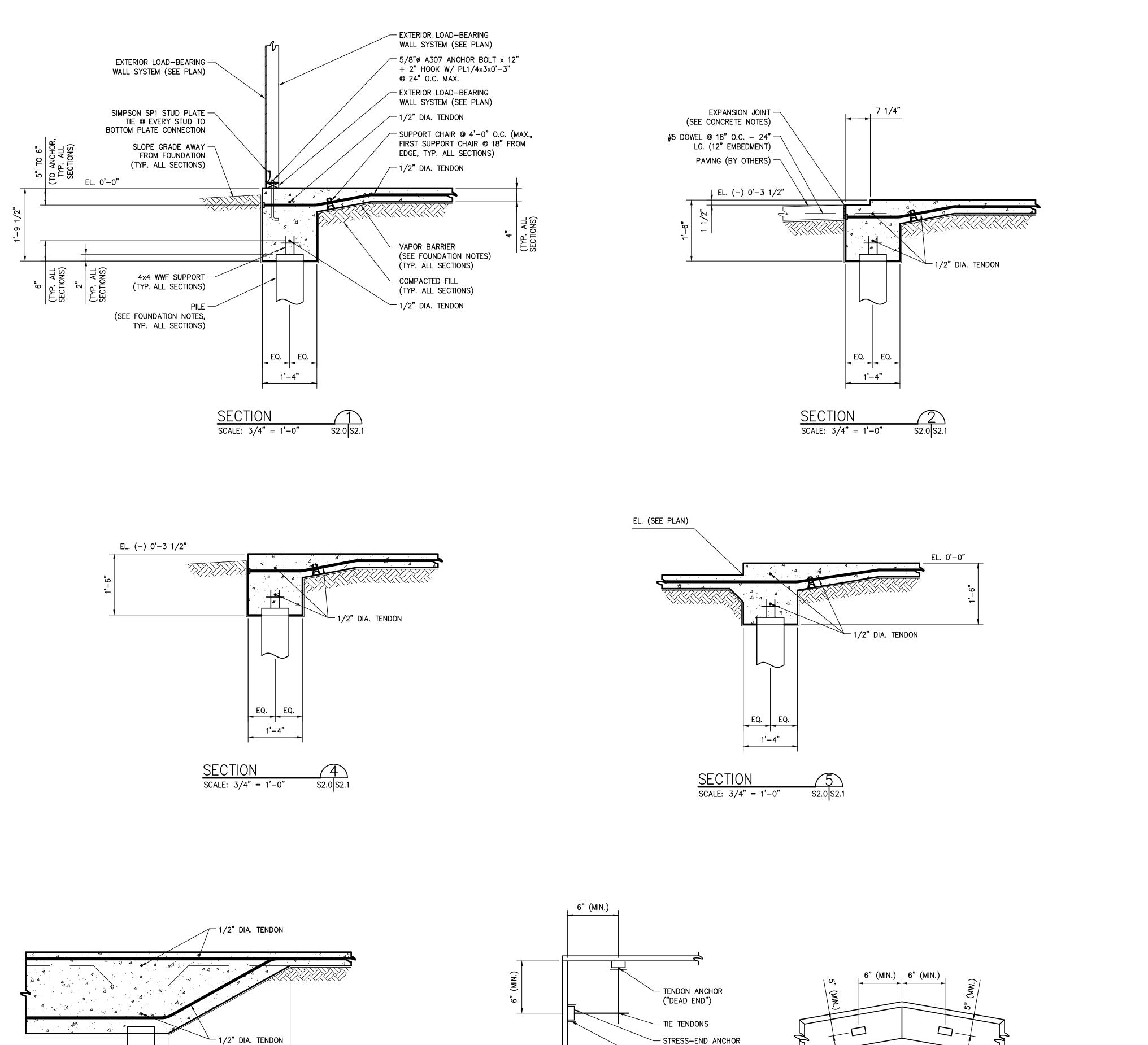
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CARPORT FOUNDATION **PLANS** 

S2.0

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6:1 SLOPE (OR 36" MIN.)

TRANSITIONAL GRADE BEAM SECTION

SCALE: N.T.S.

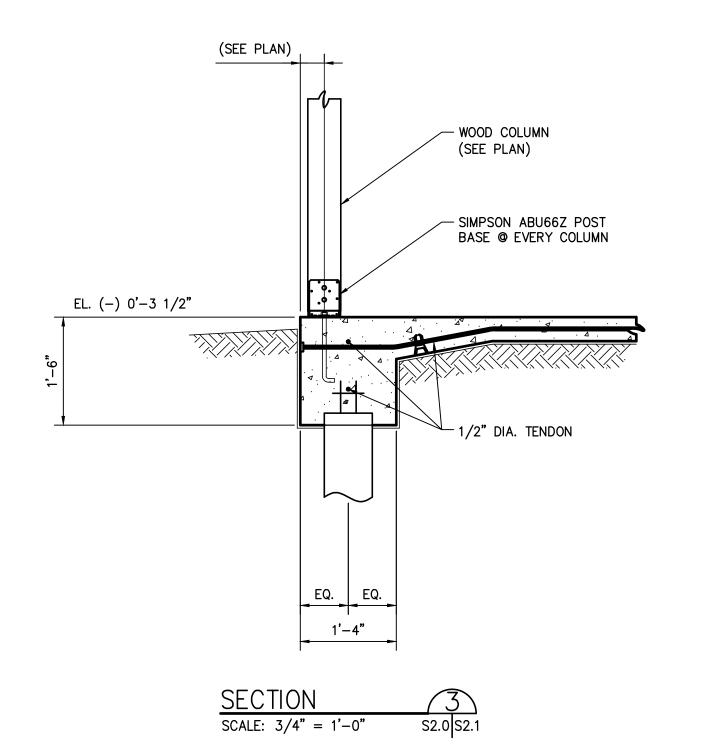
("LIVE-END")

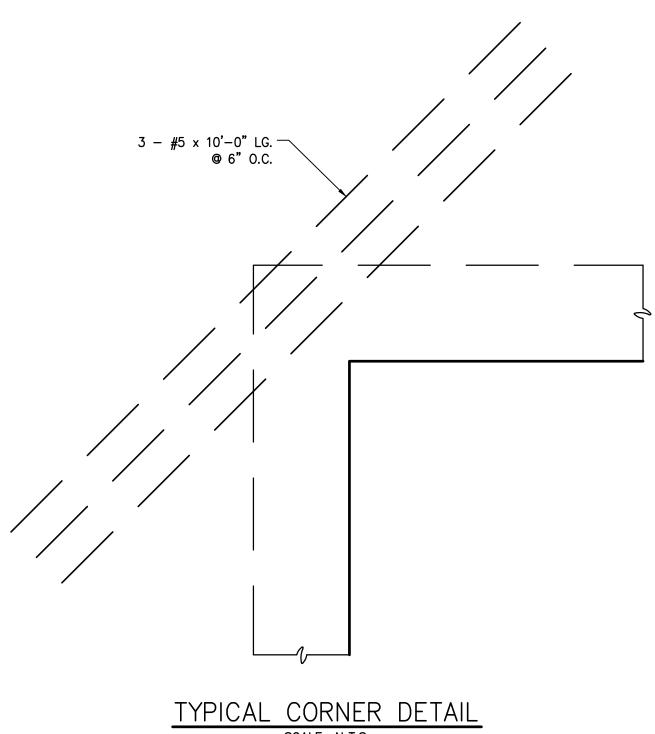
— POCKET FORMER (TYP.)

TENDON ANCHORAGE DETAIL

SCALE: N.T.S.

— 2x10 FORM BOARD (MIN., TYP.)







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